

NUMERICAL ANALYSIS OF CONCRETE COLUMNS IN FIRE

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OBJECTIVES

- The evaluation of the behavior of reinforced concrete (RC) columns in fire by means of simplified and the advanced calculation models.
- Proposal of a simplified calculation method (SimFIRc) for the analysis of RC columns in fire.
- The results of SimFIRc are compared with the ones obtained with an advanced calculation method, the finite element model SAFIR, developed by J. M. Franssen, in University of Liège, in Belgium.

CALCULATION PROCESS

- The SimFIRc model was developed for numerical analysis in fire of RC columns subjected to second order effects. It is based in the method of the nominal stiffness, described in EN 1992-1-1, for RC elements at room temperature, with the necessary adaptations for high temperatures.
- The second order effect is calculated through equation (1) which is a function of the first order moment of the critical load and the applied axial load:

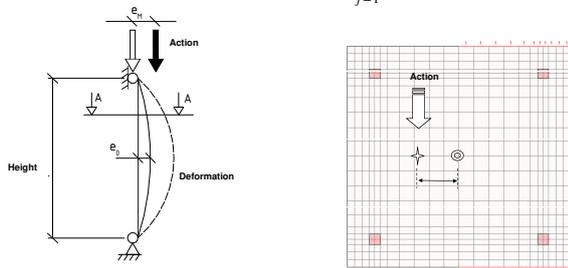
$$M_{Ed} = M_{0Ed} \cdot (1 + \beta / (N_B / N_{Ed} - 1)) \quad (1)$$

MECHANICAL MODEL

- The interaction diagram can be created with the equations of equilibrium (2) for a certain instant of time:

$$M_{Rd,fi} = \sum_{i=1}^3 b_{fi} [a_i \cdot f_{cd,fi}(\theta_i, \epsilon_{c,i})] \cdot \left[\frac{h}{2} - a_z - \frac{a_i}{2} - a_n \right] - \sum_{k=4}^6 b_{fi} [a_k \cdot f_{cd,fi}(\theta_k, \epsilon_{c,k})] \cdot \left[-\frac{a_k}{2} - a_l \right] + \sum_{j=1}^m A_{sj} \cdot f_{sy,fi}(\theta_j, \epsilon_{s,j}) \cdot z_j \quad \begin{matrix} n < i \\ l < k \end{matrix}$$

$$N_{Rd,fi} = \sum_{i=1}^6 b_{fi} \cdot a_i \cdot f_{cd,fi}(\theta_i, \epsilon_{c,i}) + \sum_{j=1}^m A_{sj} \cdot f_{sy,fi}(\theta_j, \epsilon_{s,j}) \quad (2)$$



EXAMPLES

- Figure 1 presents the loadbearing capacity obtained by the SimFIRc and SAFIR for 30, 60, 90 and 120 minutes of a 8m tall RC column, with a cross-section of 0.3 x 0.3 m², and a distance from the axis of the reinforcement bars to the surface of a=4cm, considering steel reinforcement bars of dia. 4φ12, 4φ6 and 4φ25.
- Figure 2 presents the loadbearing capacity for 30, 60, 90 and 120 minutes of a RC column, with a cross-section of 0.3x0.3m², reinforcement bars with dia. 4φ25, a=4cm, and 4, 8 and 12m tall.

CONCLUSIONS

- The formulation of EN 1992-1-1 indicated for calculation of the resistance of concrete sections at room temperature was adapted by the author¹ for high temperatures (SimFIRc), being the results at the level of the ones obtained with SAFIR.

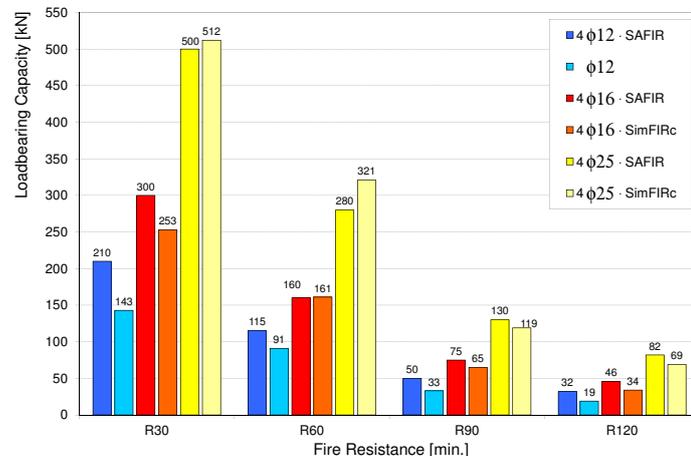


Fig. 1 –Loadbearing capacity of a RC column 8m tall for different fire resistances

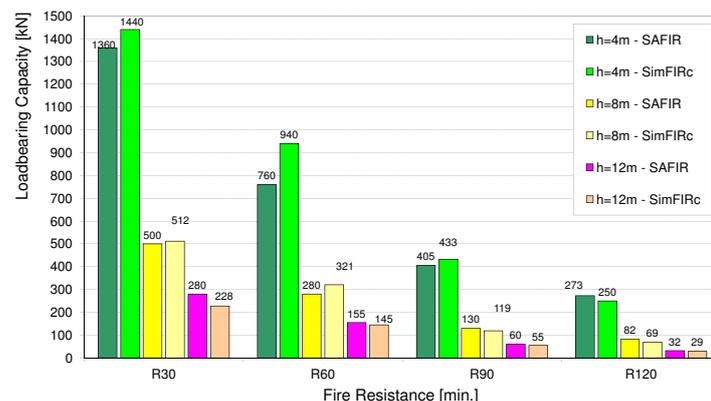


Fig. 2 –Loadbearing capacity of a RC columns with 4, 8 and 12m tall