



Integrated Fire Engineering and Response

COST action network number TU0904 in domain Transport and Urban Development

Application of Structural Fire Engineering – A.S.F.E.

29 April 2011, Prague, Czech Republic

APPLICATION OF FIRE SAFETY ENGINEERING FOR OPEN CAR PARKS IN ITALY

Emidio NIGRO, Anna FERRARO, Giuseppe CEFARELLI,
Gaetano MANFREDI, Edoardo COSENZA

*DIST – Department of Structural Engineering,
University of Naples Federico II, Naples, Italy*



Fire Safety Engineering – Open Car Parks

Italian codes

➤ Italian Prescriptive Code

✓ D.M.Int 01-02-1986



R90 for Closed Car Parks

NOT WELL DEFINED for Open Car Parks



✓ REPORT PARCHEGGI (REPORT ON ITALIAN CAR PARKS) “Approccio ingegneristico per la sicurezza strutturale in caso di incendio di parcheggi aerati realizzati con struttura di acciaio”, Final Report 2010. **Commissione per la Sicurezza delle Costruzioni di Acciaio in caso di Incendio.**



Submitted for Approval to Italian Department of Fire Brigades

European codes

➤ CEC Agreement 7215 - PP/025:

“*Demonstration of Real Fire Tests in Car Parks and High Buildings*”, by CITCM (Francia), PROFIL-ARBED Recherches (Lussemburgo) e TNO (Paesi Bassi), closed 2001.



✓ In France **09-05-2006**: “*Règlement de sécurité contre les risques d’incendie et de panique dans les parcs de stationnement couverts*” Ministère de l’Intérieur et de l’Aménagement du territoire.



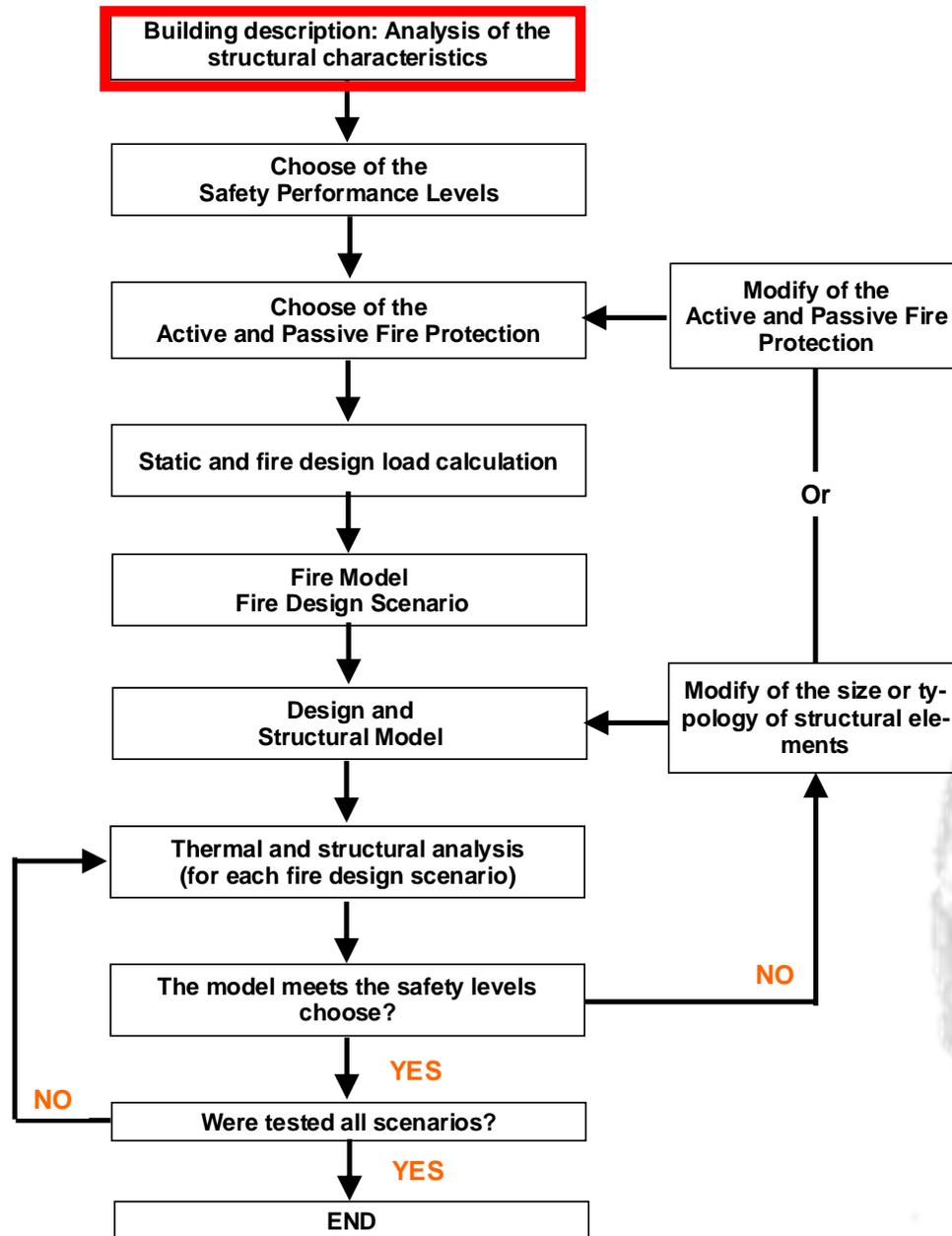
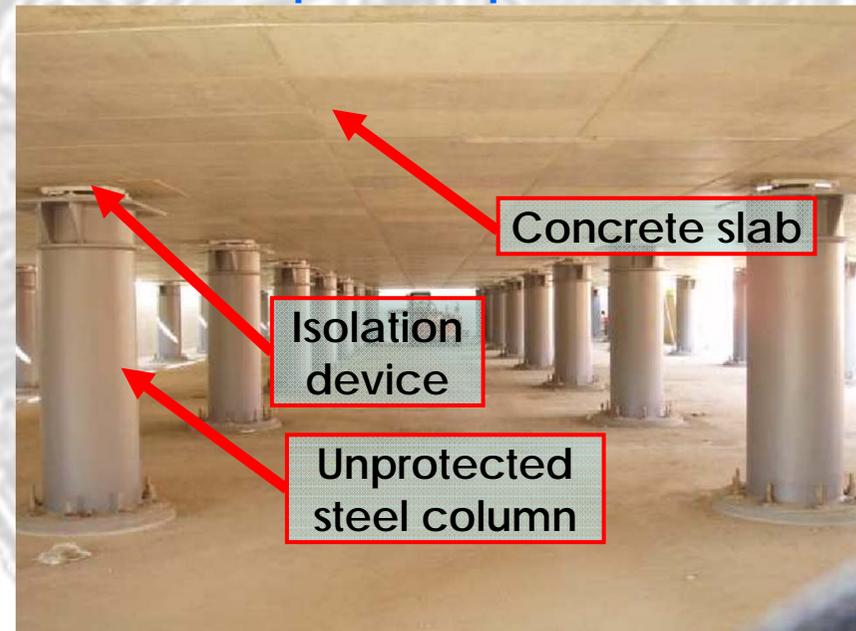
✓ Guide Lines “Parcs de stationnement en superstructure largement ventilés. Avis d’expert sur les scénarios d’incendie”, Final Report 2001 by **INERIS** (Institut National de l’Environnement Industriel et des Risques) and by CTICM (Centre Technique Industriel de la Construction Metallique).

Fire Safety Engineering – Open Car Parks

C.A.S.E. Project – L'Aquila (Italy)



Open car park



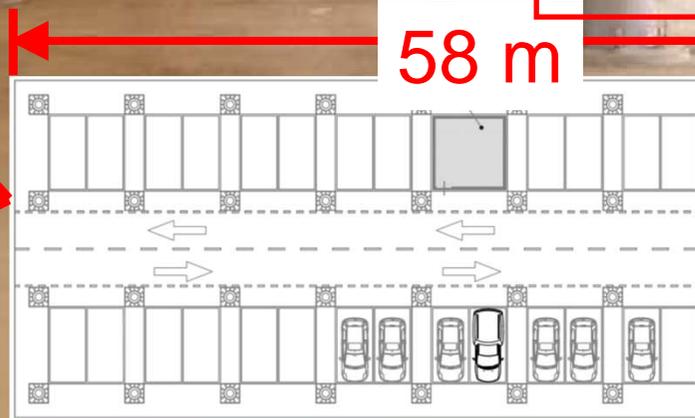
Fire Safety Engineering – Open Car Parks

Park Description:

- Below each isolation interface is located the parking area for about 34 cars; Italian prescriptive code (D. M. 01/02/1986)
- Each concrete base slab, with thickness 50 cm, is supported by steel columns (260 cm in height) with intervening seismic isolators; **R90**
- The size of the plant compartment in garage amounted to 22m × 58m; in fact, the perimeter walls, when present, are offset by 50 cm beyond the vertical projection of the plate isolated;
- The circular hollow steel sections have a capital at the top, which is designed to transfer load from the isolator unit and to allow the isolator replacement.

CLOSED CAR PARK

OPEN CAR PARK



DAMAGE

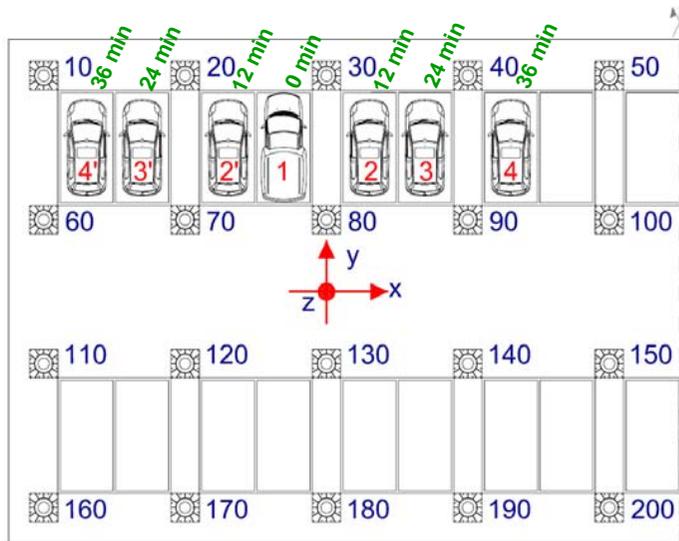
58 m

06 10 2009

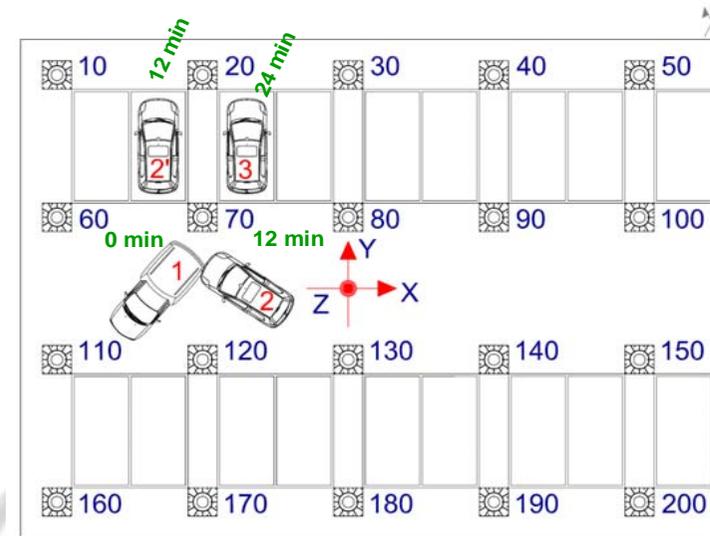
Design Fire Scenarios

Localised fire (Pre-flashover) From INERIS (2001) guideline

Fire scenario L1

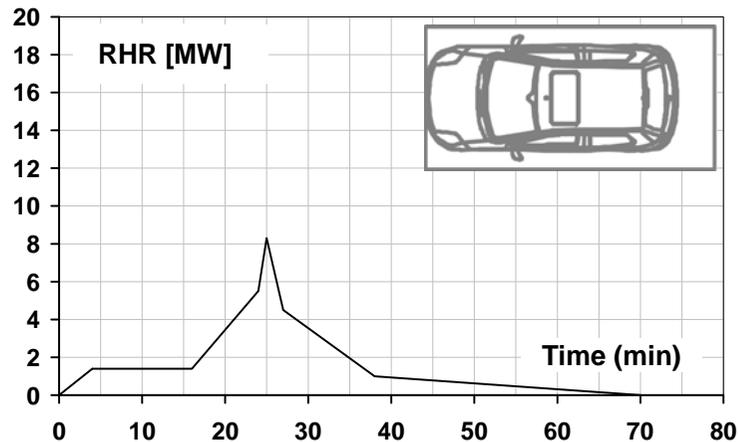


Fire scenario L2

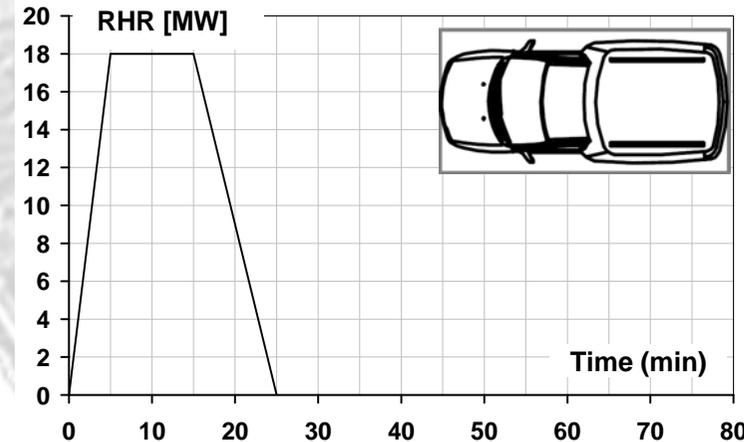


RHR curves From CEC agreement 7215-PP/025

Car (Category 3)

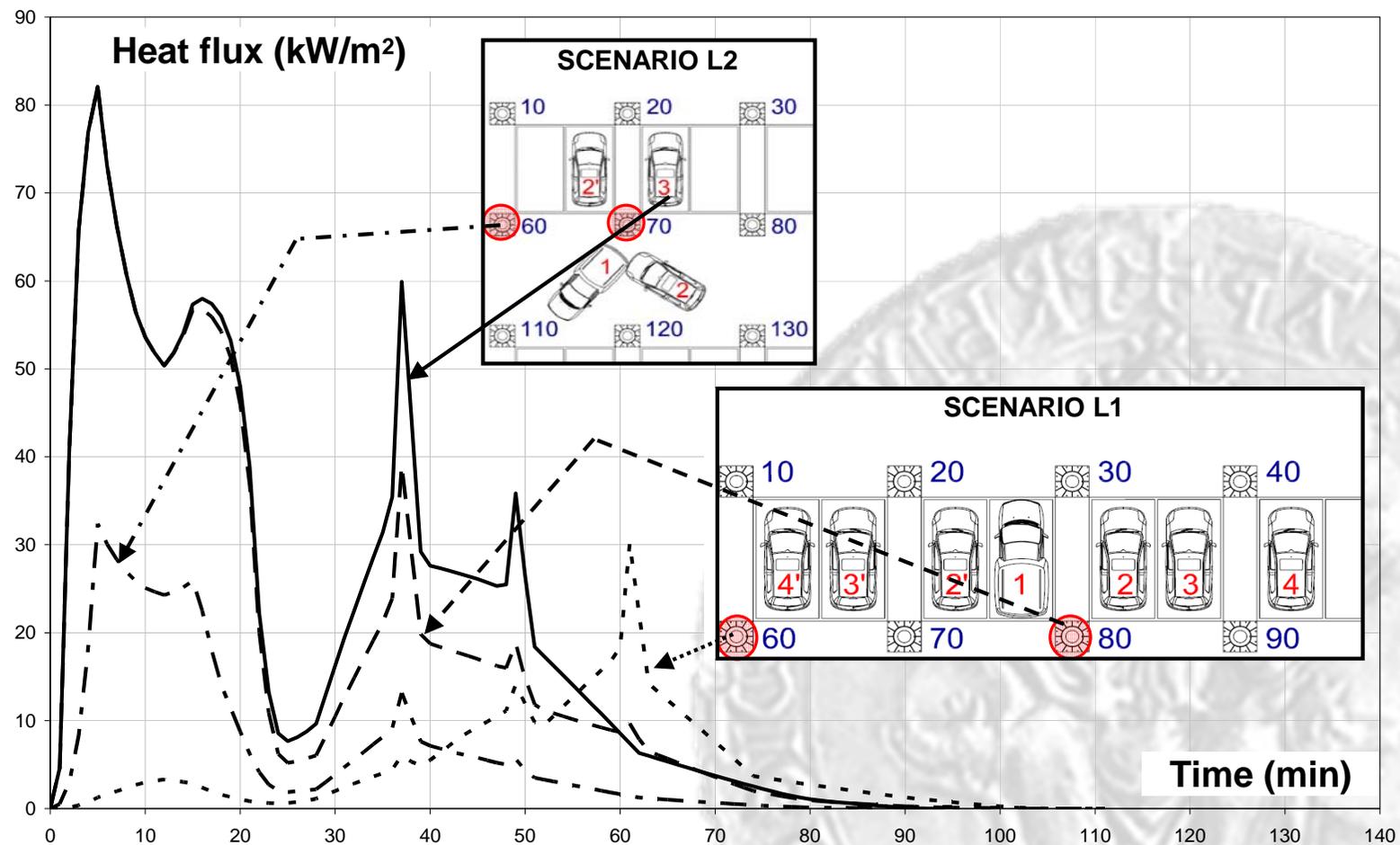


VAN



Fire model

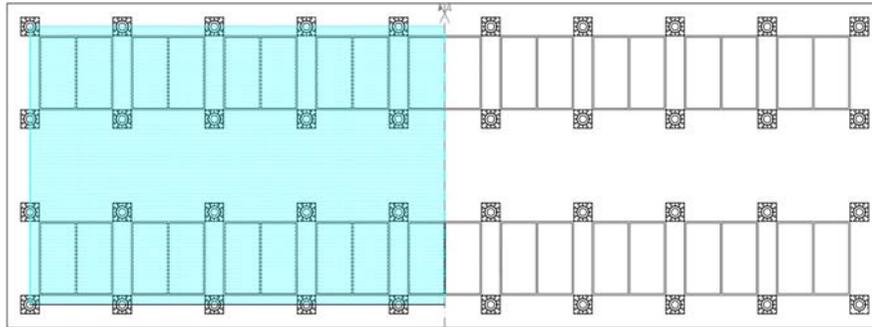
Hasemi Method From Annex C EN1991-1-2



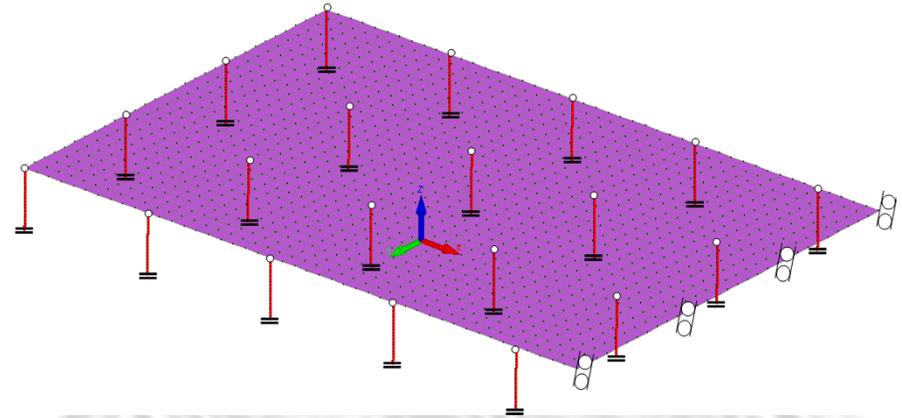
Structural models

Global analyses with non linear software SAFIR2007

Substructure

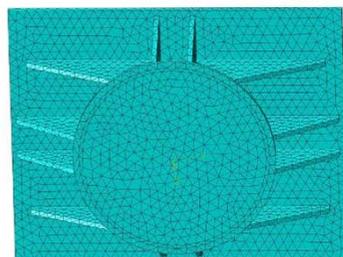
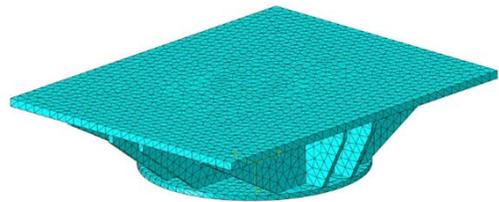


Static scheme



3D-Detailed analyses with software ABAQUS/standard

Column



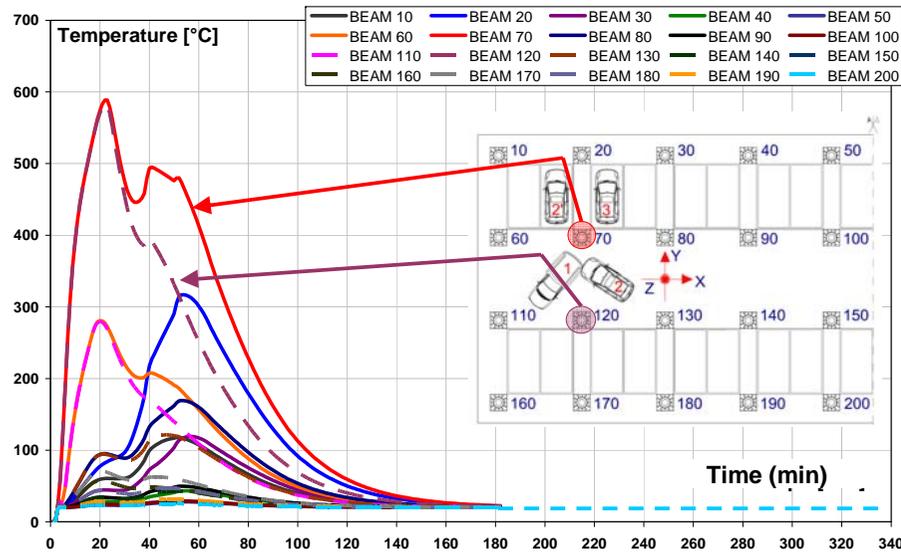
Loads on column corresponding to actions from global analysis

Performance Level 4:
Checks in terms of resistance and limitation of damage
(differential vertical displacements in the columns)

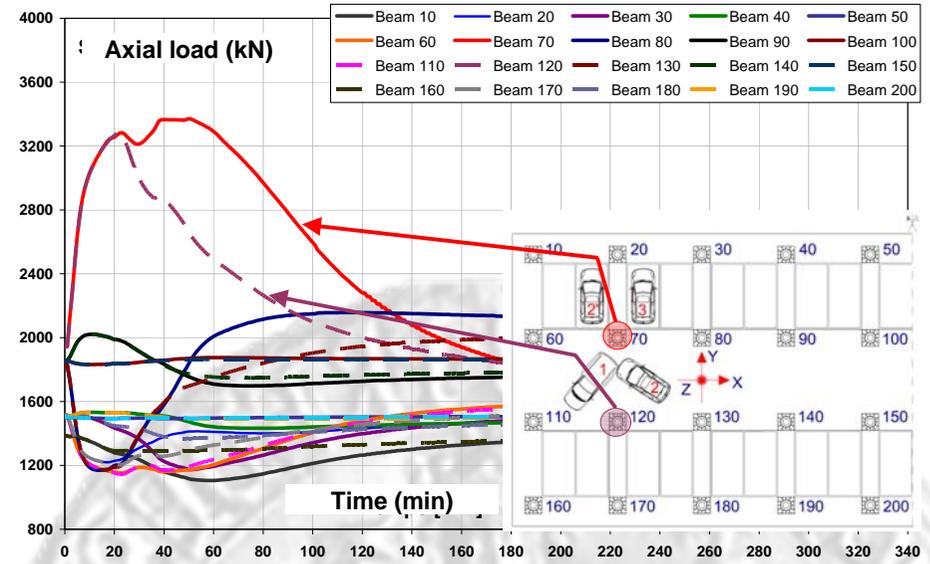
Global analyses results

Fire scenario L1

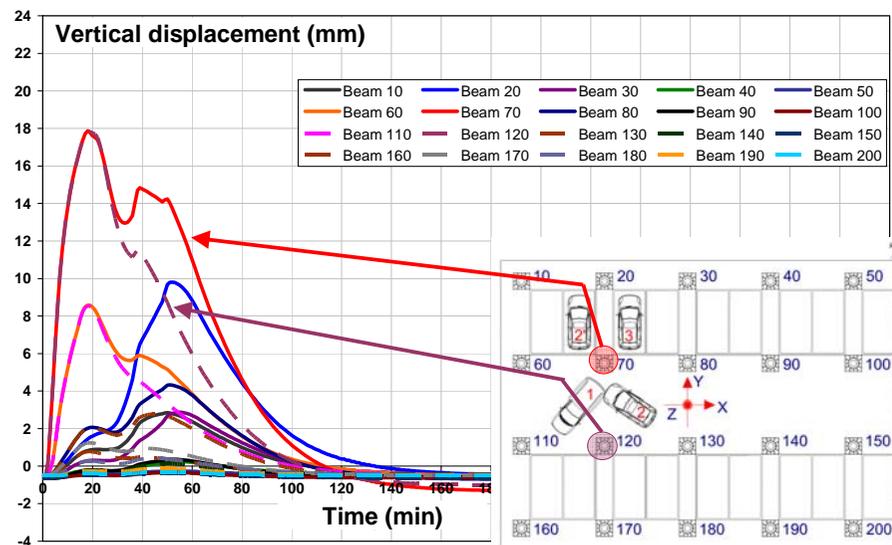
Temperatures vs time



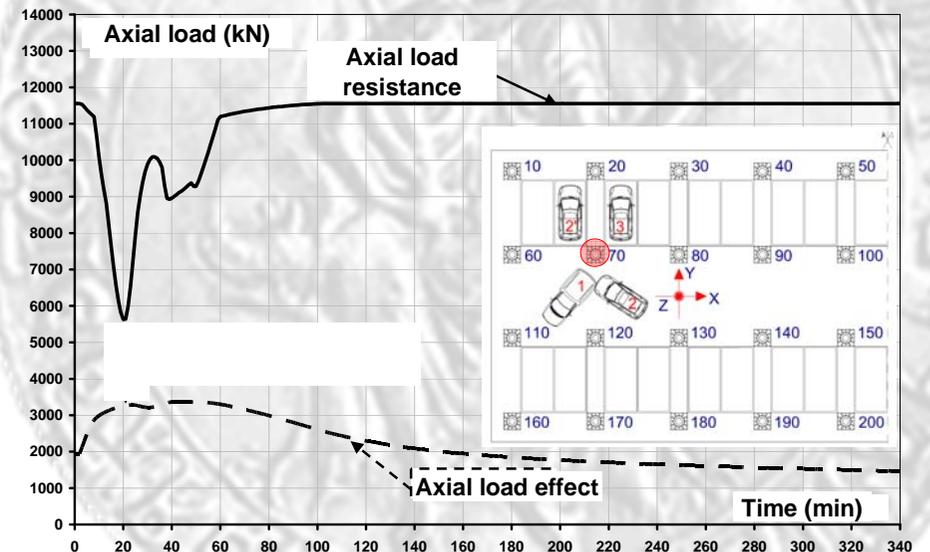
Axial loads vs time



Displacements vs time

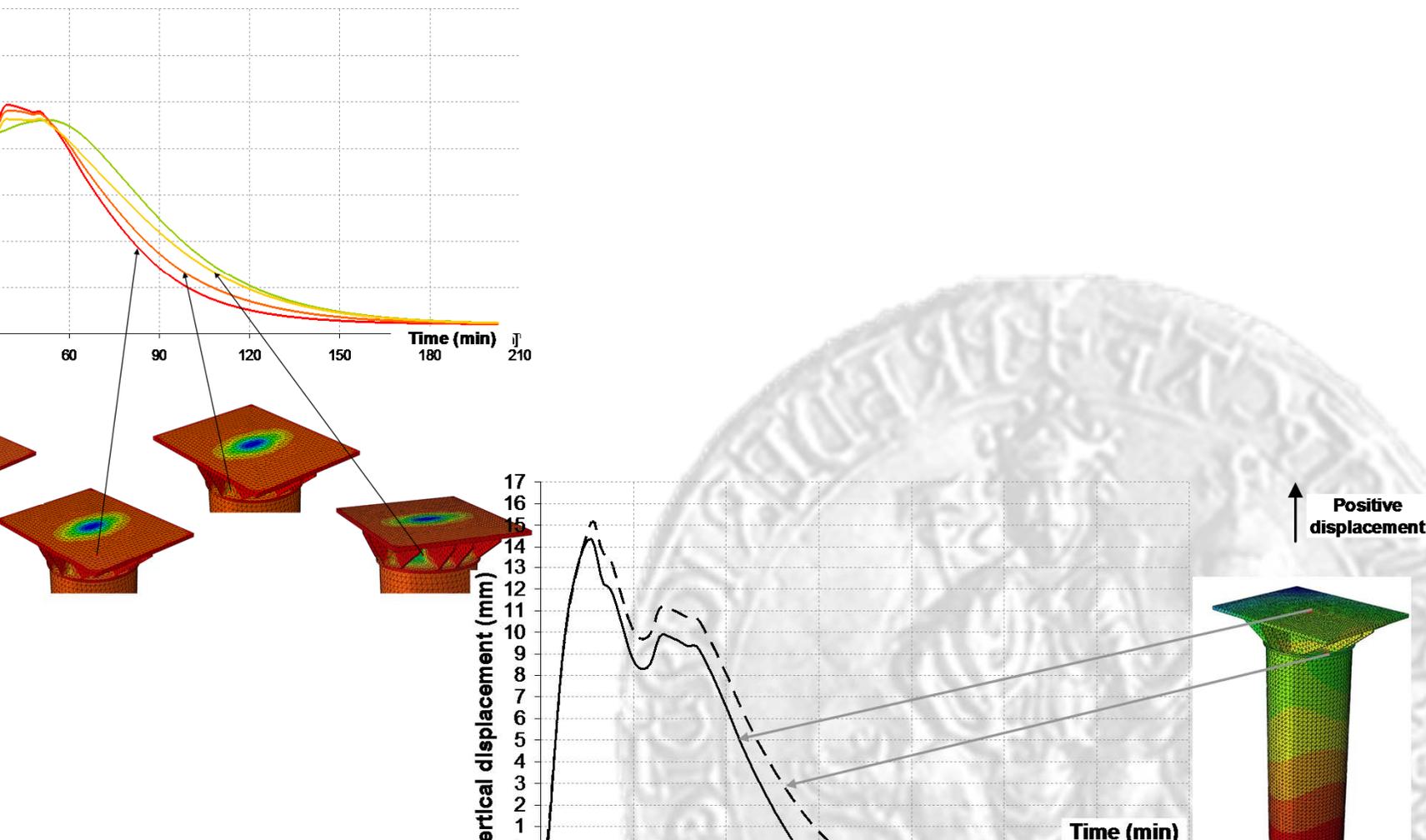


Axial load resistance vs time



Detailed analyses results

Scenario L1



Conclusions

SE application to car parks is facilitated by the informations about the probable fire scenarios provided by the European Research Project CEC Agreement 7215-PP/025 (2001) and from INERIS (2001) guideline.

Substructure extension has allowed assessing in an appropriate way both thermal field and the hyperstatic effects induced by different thermal expansions of steel columns and bending of the concrete reinforced slab.

In addition to the global analysis, for each fire scenario, in order to calculate more accurately the thermal field and stresses distribution in the capitals of the columns and to assess the possible local buckling, a detailed 3D thermo-mechanical analyses has been conducted with reference to the more detailed and heated column.

Thermo-mechanical analyses in fire situations for the described case study have proved that the structures, and in particular the steel columns, considered as fire protected, satisfy the performance level set to the design fire scenarios, also thanks to an overstrength in normal condition design.