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STUDY OF SLAB FIRE RESISTANCE ACCORDING TO EUROCODE Using different computational methods

Introduction: Eurocode 1992-1-2 includes the following alternative design methods: detailing according to tabulated data, simplified calculating method for specific types of members and general calculating method for simulating the behavior of structural members, parts of structure or the entire structure. For determining the fire resistance of reinforced concrete slab structure the tabulated data and the simplified calculating method are used. Temperature distribution in concrete cross-section is determined using FEM analysis (ANsys) and numerical solution (Nonstac) of differential equation of heat transfer. Calculated temperatures are compared with temperature profiles given in annex A. Structural response of slab cross-section is determined using "500°C" isotherm method.

Study example:

Study example of slab cross-section with thickness 200 mm is analysed. The slab is reinforced with profile 10/100 mm, concrete cover is 25 mm, distance between the reinforcement centre of gravity and exposed side of the slab is 30 mm. Slab is made of concrete C20/25 and steel B420B.

Heat exposure model – numerical analysis – Nonstac

Numerical analysis is provided using Nonstac computer program. Nonstac computer program solves numerically Fourier differential equation of one dimensional heat transfer using Runge-Kutta method

Tab. 2 Temperatures in reinforcement – NONSTAC

| | temperature dependent | | constant | | | |
|---------|-----------------------|-------|----------|-------|-------|--|
| λ | 2.0 | 1.36 | 2.0 | 1.36 | 2.0 | |
| density | 2400 | 2400 | 2400 | 2400 | 2300 | |
| Time | Temp. | Temp. | Temp. | Temp. | Temp. | |
| min | C | С | С | C | С | |
| 0 | 20 | 20 | 20 | 20 | 20 | |
| 30 | 331 | 291 | 370 | 332 | 340 | |
| 60 | 504 | 465 | 574 | 533 | 540 | |
| 90 | 604 | 569 | 687 | 647 | 654 | |
| 120 | 670 | 643 | 765 | 726 | 732 | |

Heat exposure model – input data according to Eurocode 2: Heat conductivity λ :

Heat concuctivity of concrete is assumed as temperature dependant and is given with upper (1) and lower (2) limit value:

 $\lambda_{c} = 2.0 - 0.2451 \, (100 + 0.0107 (\theta/100)^{2})^{2} \, (1)$

$$\lambda_c = 1.36 - 0.136 \, (100)^2 \, (2)^2$$

Concrete specific heat c:

Basic value of concrete specific heat is 900 (J/kg.K). With increasing temperature concrete specific heat growths to the value 1100 (J/kg.K). It is possible to take into account the initial humidity with local significant increasing of specific heat in the temperature interval 100°C-115°C. In this study example zero initial humidity is assumed on the safe side.

Concrete density ρ :

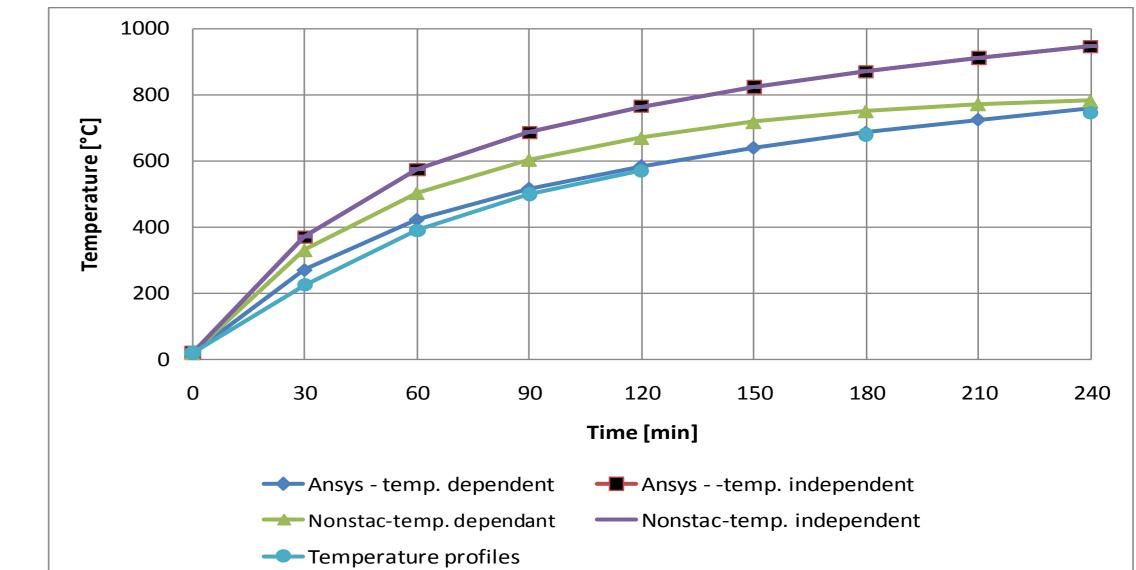
According to P ENV version of Eurocode 2 it is recommended to use the value 2300 kg/m³, without the temperature dependence. According to Eurocode the density is given as a function of temperature, but without the recommended value for the initial density. The common concrete density is given also in Eurocode 1, part 1-1 (2004) with the value 2400 kg/m³.

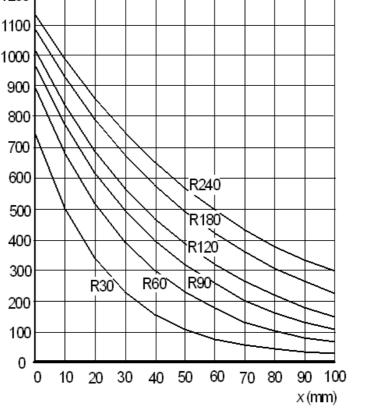
Heat exposure model – temperature profiles – Annex A of EC 2:

As for the slab structures, the temperature profile is given only for the slab of thickness 200 mm. The temperatures were appointed for lower value of heat conductivity, specific heat with assuming the initial humidity 1.5% and the density 2400 kg/m³.

| θ(C | ;) | | |
|--------|----|--|--|
| 1200 t | | | |
| 12001 | | | |

Heat exposure model – comparing the temperatures





Heat exposure model – FEM analysis – Ansys

The FEM analysis is provided using ANsys computer program. In the Table 1 there are sequenced temperatures in reinforcement for different input data.

Tab. 1 Temperatures in reinforcement – Ansys

| | temperature dependent | | | constant | | | |
|---------|-----------------------|-------|-------|----------|-------|-------|--|
| λ | 2.0 | 1.36 | 2.0 | 2.0 | 1.36 | 2.0 | |
| density | 2400 | 2400 | 2300 | 2400 | 2400 | 2300 | |
| Time | Temp. | Temp. | Temp. | Temp. | Temp. | Temp. | |
| min | С | С | С | С | С | С | |
| 0 | 20 | 20 | 20 | 20 | 20 | 20 | |
| 30 | 271 | 245 | 277 | 370 | 332 | 367 | |
| 60 | 423 | 402 | 430 | 574 | 533 | 574 | |
| 90 | 517 | 502 | 524 | 687 | 647 | 689 | |

Structural response - fire resistance in time 90 minutes

| | | | Profiles | Ansys | Ansys | Nonstac |
|-----------------------------------|--------------------|-------------------|----------|-------|-------|---------|
| Heat conductivity - temp. dep. | λ | kJ/kg.K | | 2.0 | 2.0 | 2.0 |
| Density – temp. dep. | ρ | kg/m ³ | | 2400 | 2300 | 2400 |
| Temperature in reinforcement, | θ_{R} | C | 500 | 517 | 524 | 604 |
| Steel strength | f _{vd,fi} | Mpa | 328 | 305 | 232 | 193 |
| B. moment - capacity | | kNm/m | 42 | 39 | 30 | 25 |
| | | | 32 | 32 | 32 | 32 |
| Assessment | , | | OK | OK | X | X |

Conclusion:

•In the paper the study example of reinforced concrete slab is analyzed. Temperature in reinforcement is settled using temperature profiles and Ansys and Nonstac software.

•Input data for thermal analysis are not given strictly.

•Table fire resistance of analyzed slab is 90 minutes.

•As the simplified method is more advanced then detailing according to tabulated data, it is expected that resulting fire resistance of slab structure gives more favorable values

•This study would like to point out that in some cases design

