

COST TU0904
Malta, 11-13/4/2012

Training course for young researchers

Overview of the development of fire engineering in research and practice

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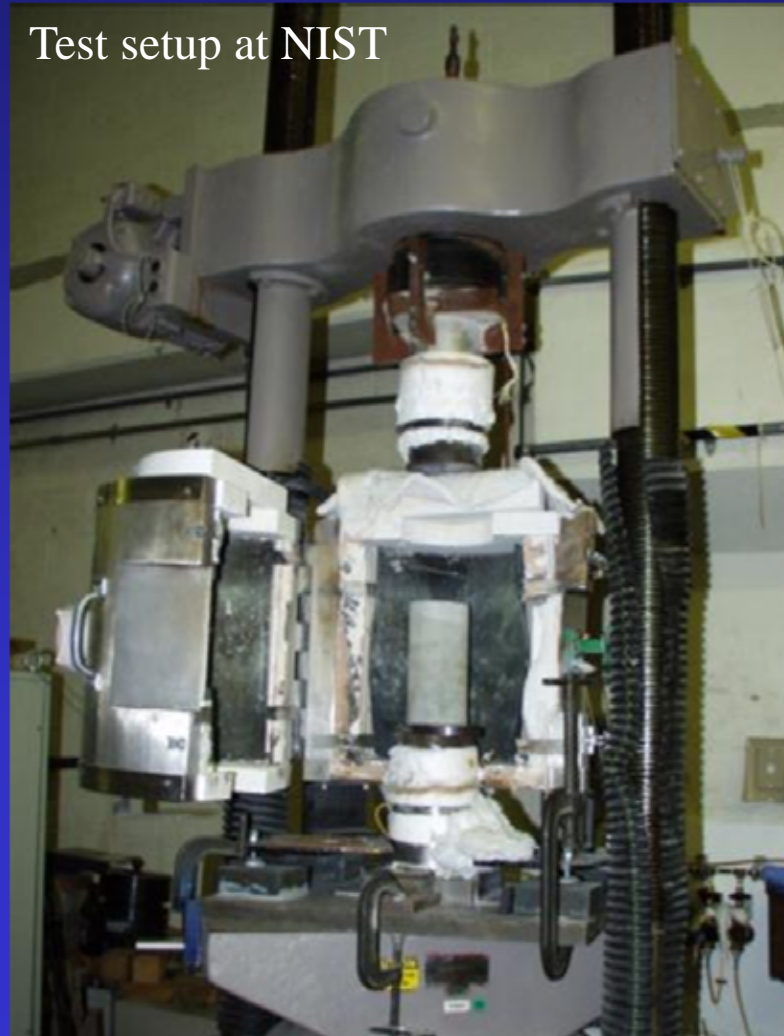
Various methods for determining the fire resistance.

- 1. Experimental Tests**
- 2. Tabulated data**
- 3. Simple calculation models**
- 4. Advanced calculation models**

Method 1 : Experimental testing

➤ Testing specimens for material behaviour

Test setup at NIST



Method 1 : Experimental testing

➤ Testing material behaviour

➤ Standard fire tests.

- Circumstantial disadvantages: cost, delays, limited # of facilities.
- Real disadvantages: only elements, size of the element, boundary conditions, variability.



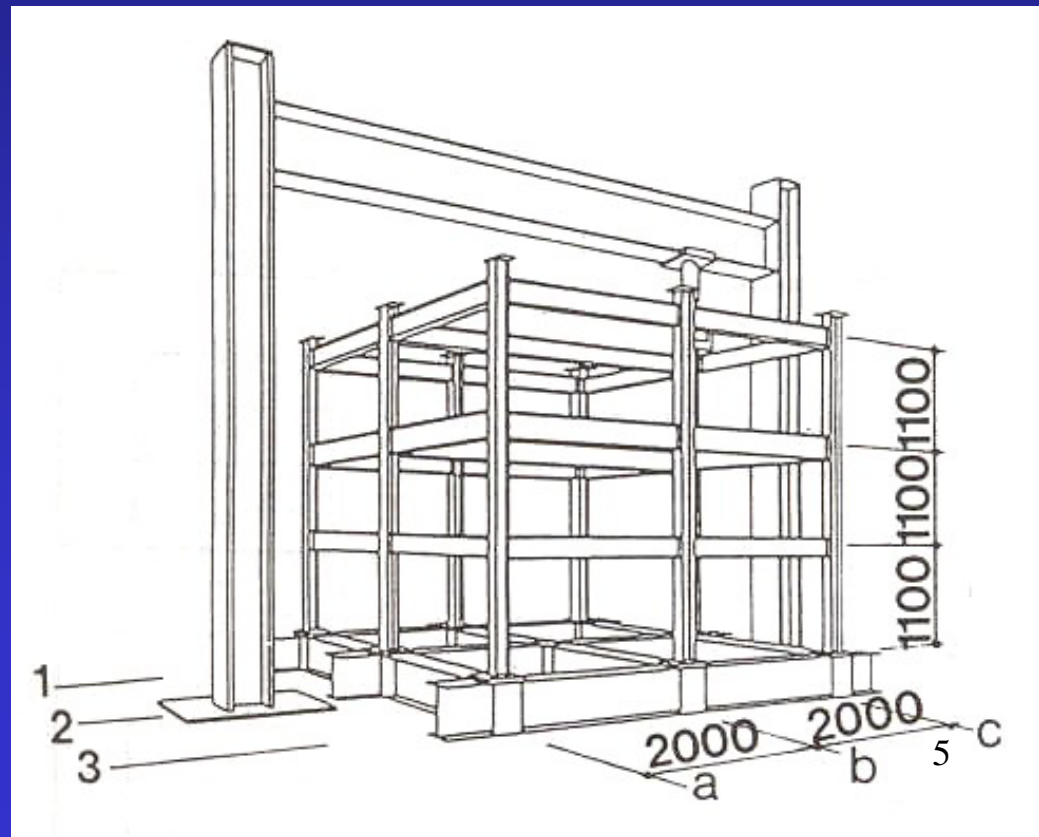
Method 1 : Experimental testing

- Testing material behaviour
- Standard fire tests
- **Small scale fire tests**

Steel: OK

Hydral materials: ???

Picture from Nakamura et al.,
1st IAFSS, Gaithersburg, 1985



Method 1 : Experimental testing

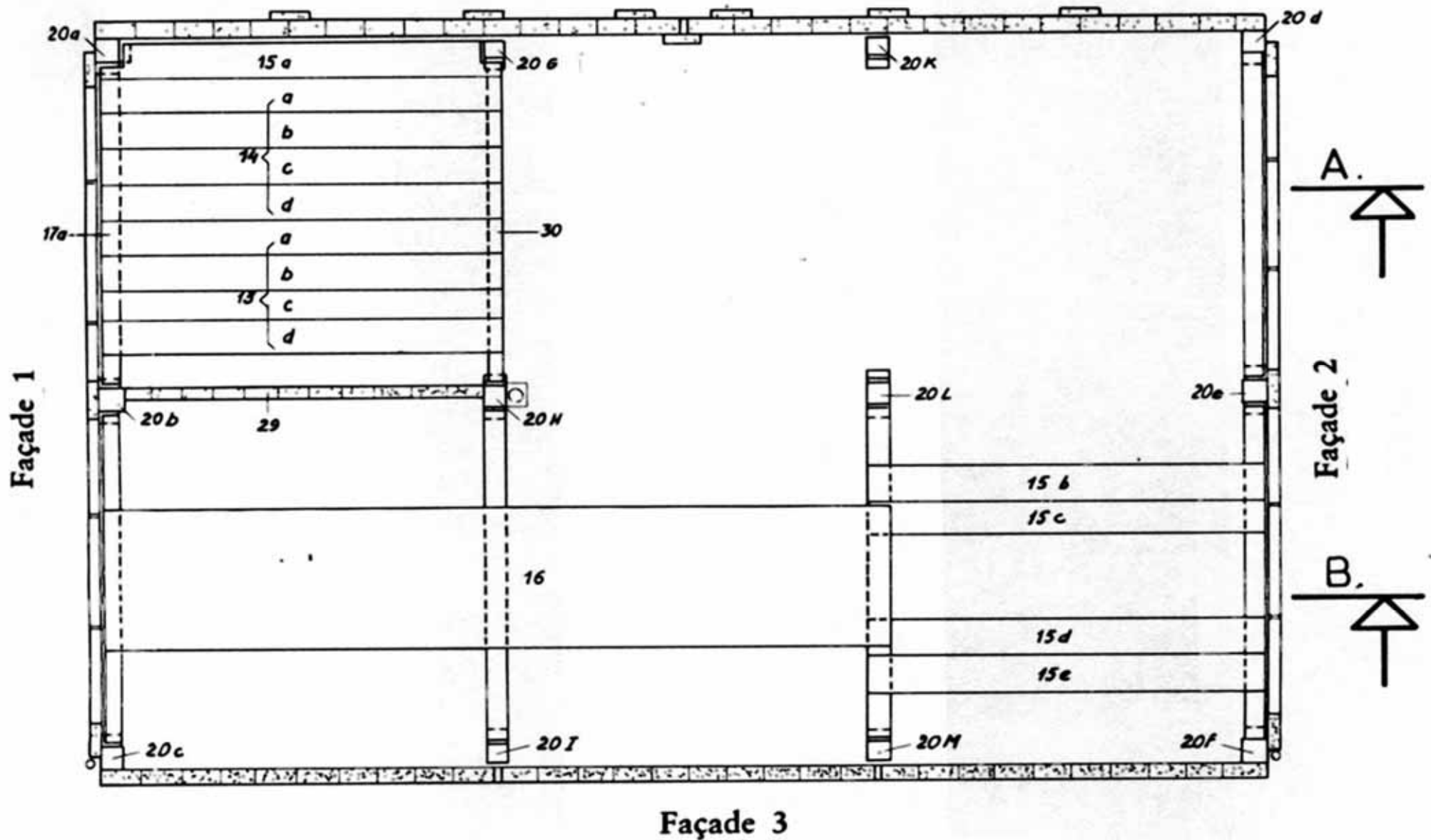
- Testing material behaviour
- Standard fire tests
- Small scale fire tests
- **Large scale fire tests**

Rare - Local fires - Observations more than research



Concrete building- Gent

Façade 4







⇒ **Experimental testing is used mainly in research.**

⇒ **Experimental testing will remain forever.**

- **Verification of basic hypotheses used in calculation models**
- **Integrity criteria in separating elements**



Method 2 : Tabulated data

Definition: presentation, in simple form, of results obtained by other methods.

Standard fire resistance	Minimum dimensions (mm)			
	Slab thickness h_s	Axis-distance a		
		One way	Two way	
			$l_y/l_x \leq 1.5$	$1.5 < l_y/l_x \leq 2$
1	2	3	4	5
REI 30	60	10*	10*	10*
REI 60	80	20	10*	15*
REI 90	100	30	15*	20
REI 120	120	40	20	25
REI 180	150	55	30	40
REI 240	175	65	40	50

l_y and l_x are the spans of a two-way slab where l_y is the longer span.
For prestressed slabs the increase of axis distance should be noted.

The axis distance a in Column 4 and 5 for two way slabs relate to slabs supported on all four edges. Otherwise, they should be treated as one-way spanning slabs.

* Normally the cover required at room temperature will control

Method 2 : Tabulated data

Reinforcement ratio $\omega = 0.50$; Eccentricity $e \leq 200$ mm

Standard fire resistance	λ	Column width b_{min} / axis distance a			
		$n = 0.15$	$n = 0.30$	$n = 0.50$	$n = 0.70$
R30	30	150/25*	150/25*	250/35:300/25*	500/40:550/25*
	40	150/25*	150/30:200/25*	300/35:450/25*	550/30
	50	150/25*	200/30:250/25*	400/40:500/25*	550/50:600/40
	60	150/25*	200/35:300/25*	450/50:550/25*	(1)
	70	150/25*	250/40:400/25*	500/40:600/30*	(1)
	80	150/25*	300/40:500/25*	550/50:600/40*	(1)
R 60	30	150/30:200/25*	200/40:450/25*	450/50:550/30	550/50:600/40
	40	150/35:250/25*	250/40:500/25*	500/40:550/35	600/60
	50	200/35:300/25*	300/45:550/25*	500/55:550/40	(1)
	60	200/40:500/25*	400/40:600/30	550/50:600/45	(1)
	70	200/40:550/25*	500/40:550/35	600/60	(1)
	80	250/40:600/25*	500/40:600/35	(1)	(1)
R 90	30	250/40:450/25*	300/50:500/25	500/55:600/40	600/80
	40	200/50:500/25*	350/50:550/35	550/60:600/50	(1)
	50	250/45:550/25*	500/45:550/40	600/60	(1)
	60	250/50:550/30	500/50:550/45	600/80	(1)
	70	300/50:550/35	550/50:600/45	(1)	(1)
	80	350/50:600/35	550/60:600/50	(1)	(1)

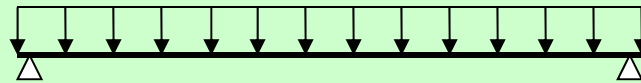
* Normally the cover at room conditions will control
 (1) Requires a width greater than 600 mm.

Method 3 : Simple calculation models

Definition: Method based on global equilibrium conditions.

$$M_{\max} \leq R_d$$

$$\frac{qL^2}{8} \leq W_{pl} f_y$$

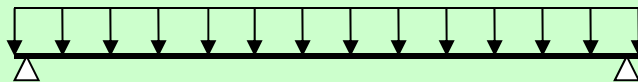


Method 3 : Simple calculation models

- Extrapolations of similar methods used at room temperature
- Can be used « by hand »
- One method for each material/member type.
- Not well suited for complex structures.

=> Used for real projects.

$$\text{At } 20^{\circ}\text{C} : \frac{q_d L^2}{8} \leq W_{pl} f_y$$



$$\text{At high temperature : } \frac{q_{d,fi} L^2}{8} \leq W_{pl} f_y (T)$$

Method 4 : Advanced calculation models

Definition: Based on principles of structural mechanics or of heat transfer (local equations).

$$\frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xy}}{\partial y} + \frac{\partial \sigma_{xz}}{\partial z} + F_x = 0$$

$$\frac{\partial \sigma_{yx}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} + \frac{\partial \sigma_{yz}}{\partial z} + F_y = 0$$

$$\frac{\partial \sigma_{zx}}{\partial x} + \frac{\partial \sigma_{zy}}{\partial y} + \frac{\partial \sigma_{zz}}{\partial z} + F_z = 0$$

$$\lambda \left(\frac{\partial^2 T}{\partial x^2} + \frac{\partial^2 T}{\partial y^2} + \frac{\partial^2 T}{\partial z^2} \right) + Q - c\rho \frac{\partial T}{\partial t} = 0$$

Method 4 : Advanced calculation models

- **Finite differences, finite elements, boundary elements.**
- **Require a computer (numerical calculation models).**

Three different families of software:

1. '*My Ph.D.*' software

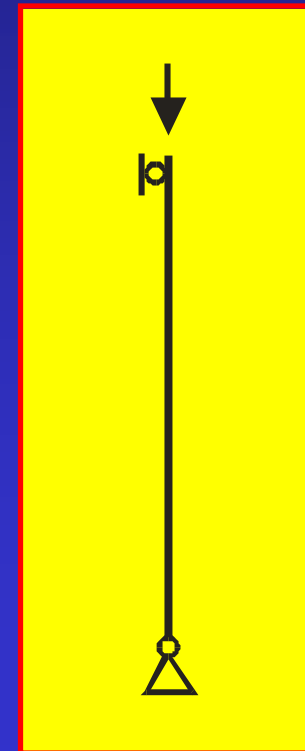
- One author (university)



Three different families of software:

1. '*My Ph.D.*' software

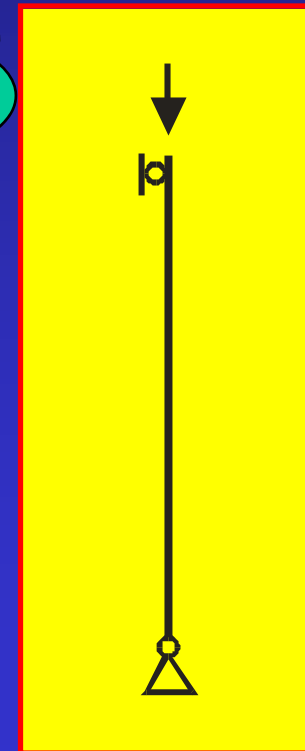
- One author (university)
- Limited field of application



Three different families of software:

1. '*My Ph.D.*' software

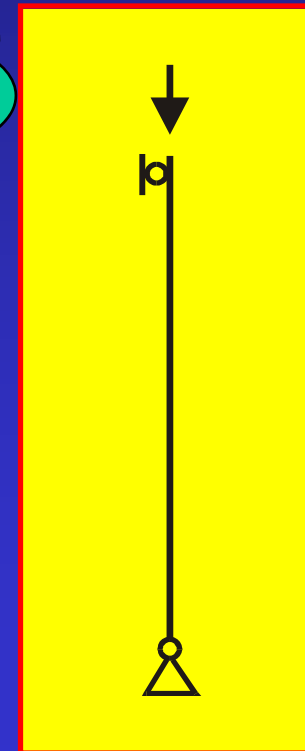
- One author (university)
- Limited field of application
- Limited availability



Three different families of software:

1. '*My Ph.D.*' software

- One author (university)
- Limited field of application
- Limited availability
- Limited durability



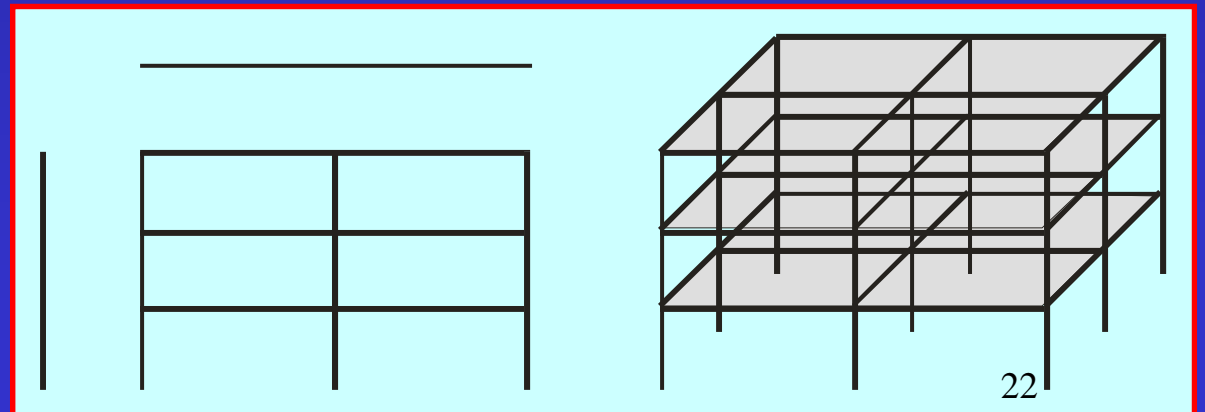
Three different families of software:

1. '*My Ph.D.*' software
2. **Dedicated software (VULCAN, SAFIR,...)**
 - **From a group (University)**



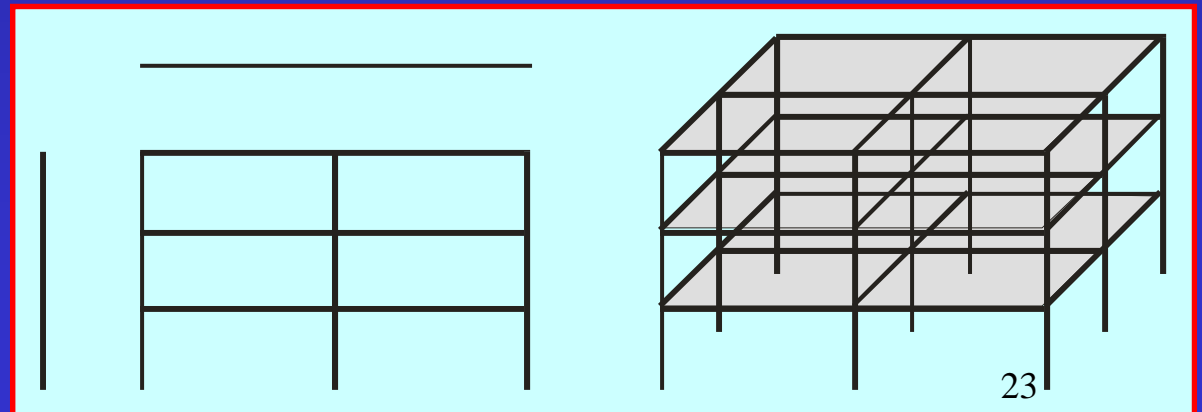
Three different families of software:

1. *'My Ph.D.'* software
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 - Wider field of application



Three different families of software:

1. *'My Ph.D.'* software
2. **Dedicated software (VULCAN, SAFIR,...)**
 - From a group (University)
 - Wider field of application
 - Become available now



Three different families of software:

1. '*My Ph.D.*' software
2. Dedicated software (VULCAN, SAFIR,...)
3. Commercial software (ANSYS, ABAQUS,...)
 - Widely distributed, used and validated
 - Price !!!
 - Nice graphics

+++ or --- ?

What can we model and what should we test?

Which material can we model?

A priori, all of them...
if we have the properties.

Which properties?

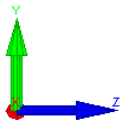
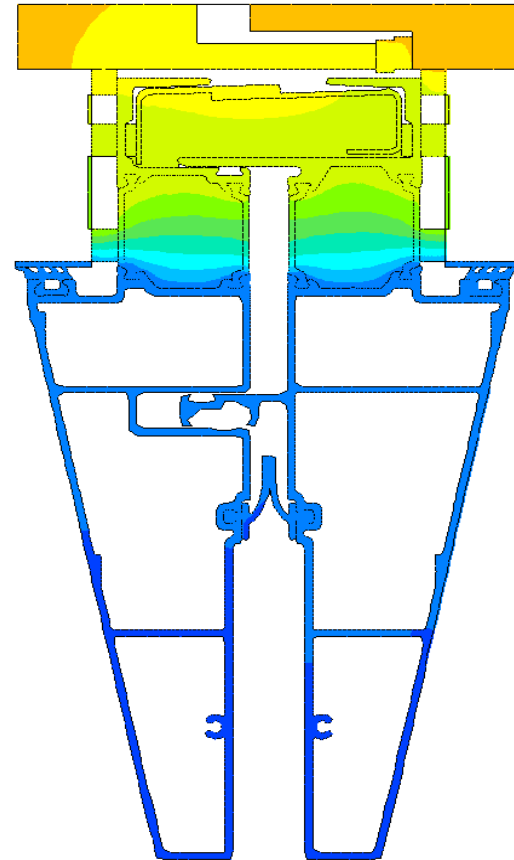
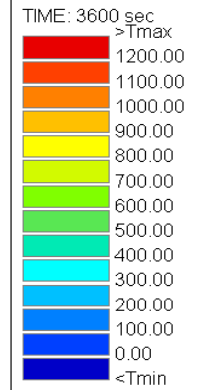
Properties of the material?

No. Properties of the model.

=> Know the limits of your model.

FILE: final
NODES: 5519
ELEMENTS: 8566

CONTOUR PLOT
TEMPERATURE PLOT



Window frame (courtesy: Permasteelisa)

What can we model and what should we test?

Which structure can we model?

A priori, none of them...

except if we made a test before on a similar structure.

examples

1) Composite floor on corrugated steel sheets

Diamond 2009.a.6 for SAFIR

FILE: T100_120

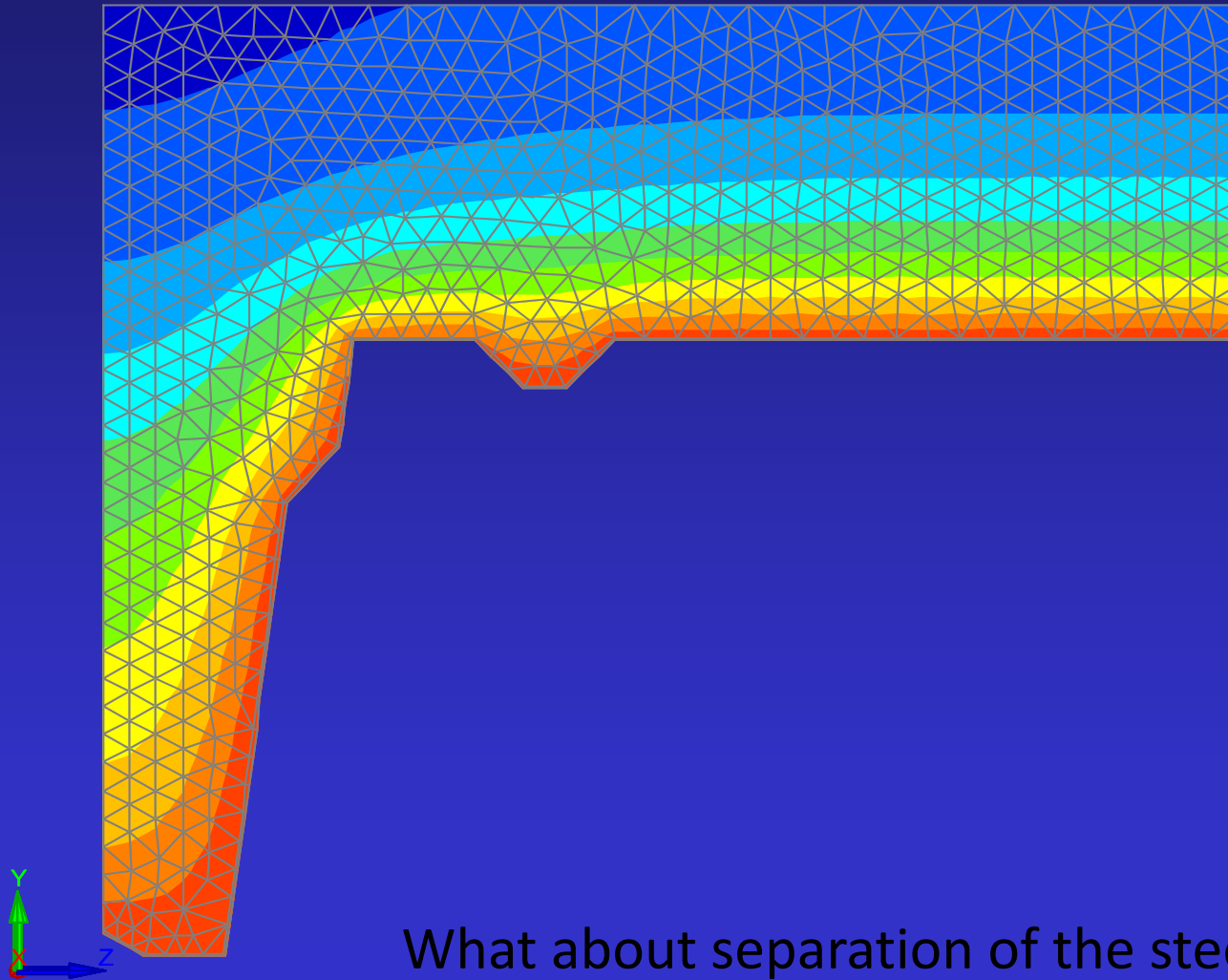
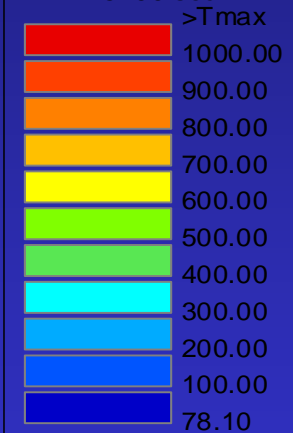
NODES: 765

ELEMENTS: 1323

SOLIDS PLOT

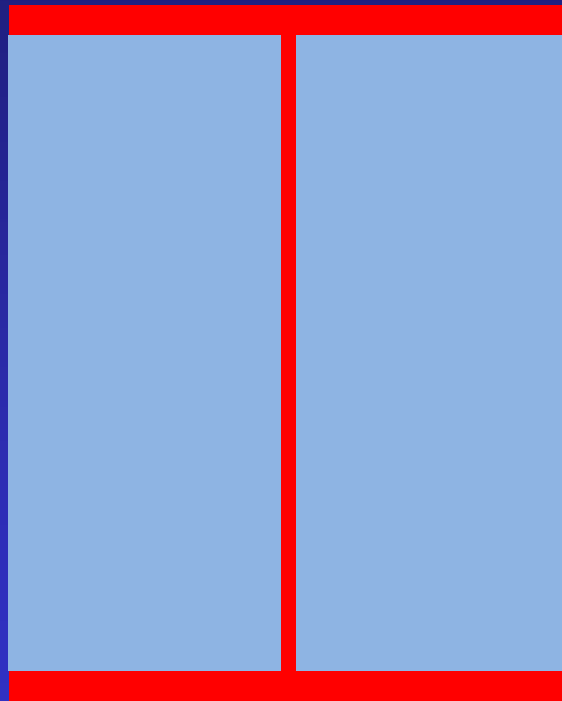
TEMPERATURE PLOT

TIME: 5400 sec

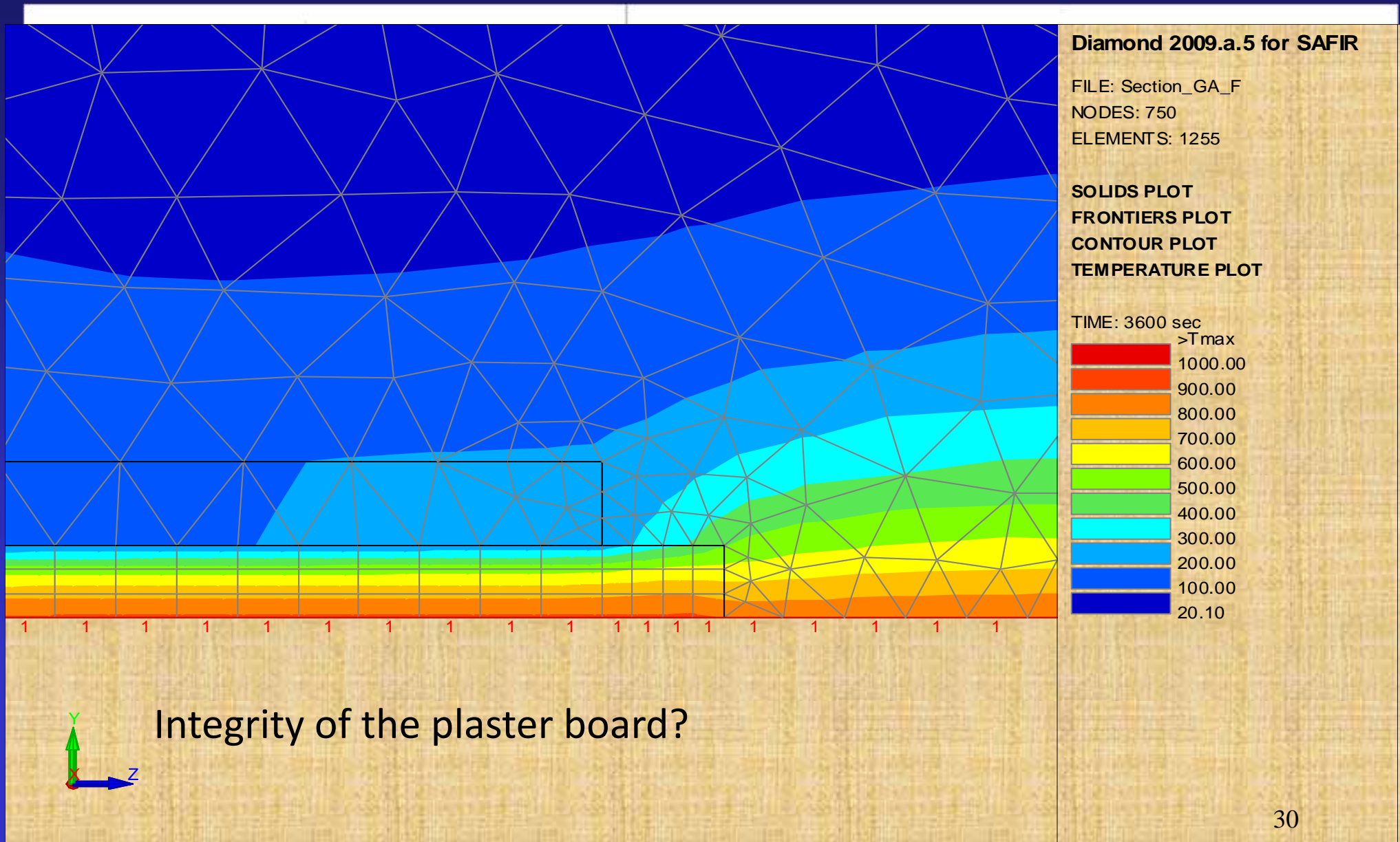


What about separation of the steel sheet from the slab?

2) Composite steel concrete columns

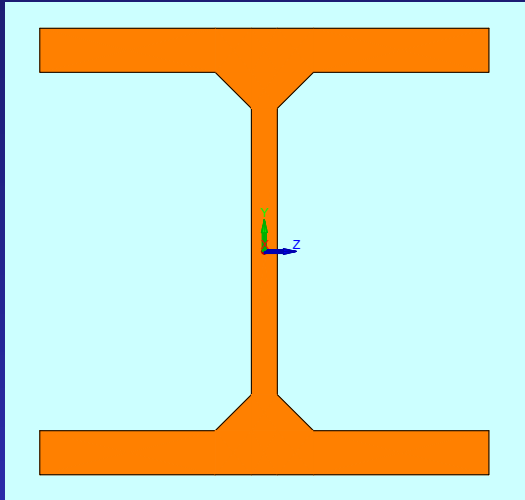


3) Steel plate covered by a plaster board

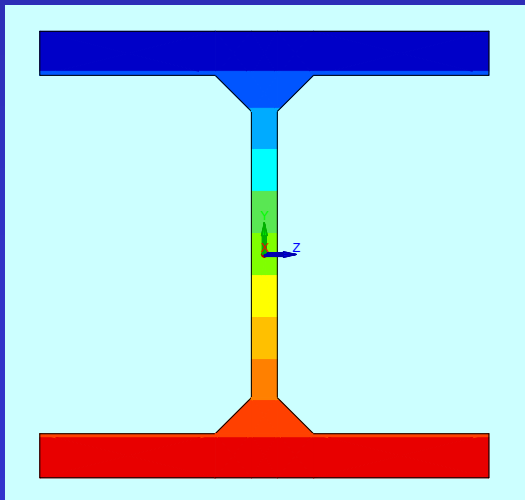


Yesterday

Uniform temperature

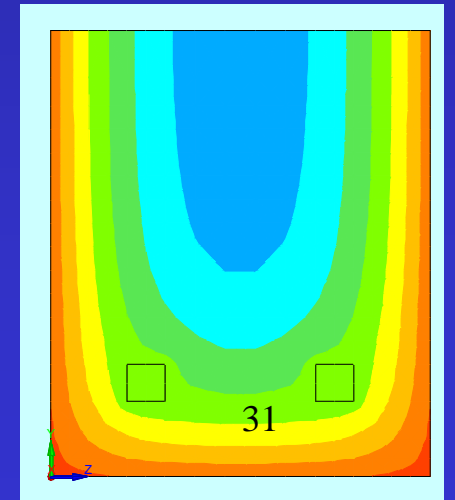
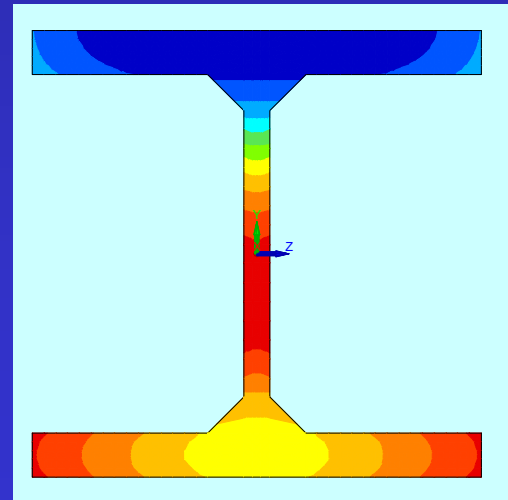
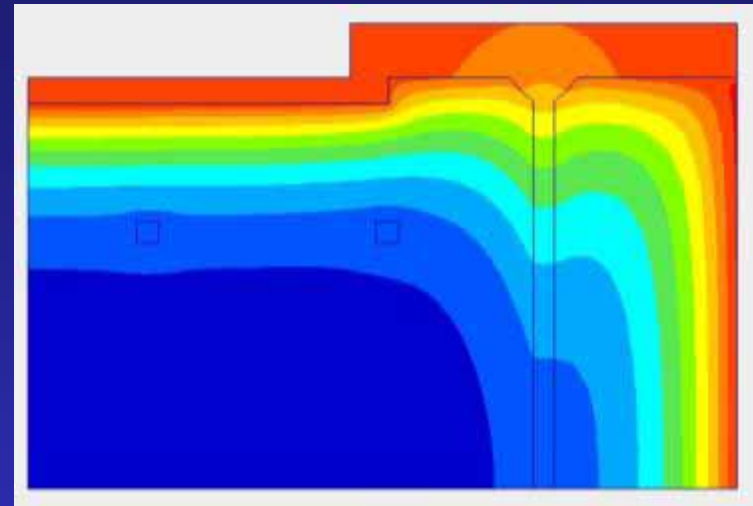


Linear gradient



Today

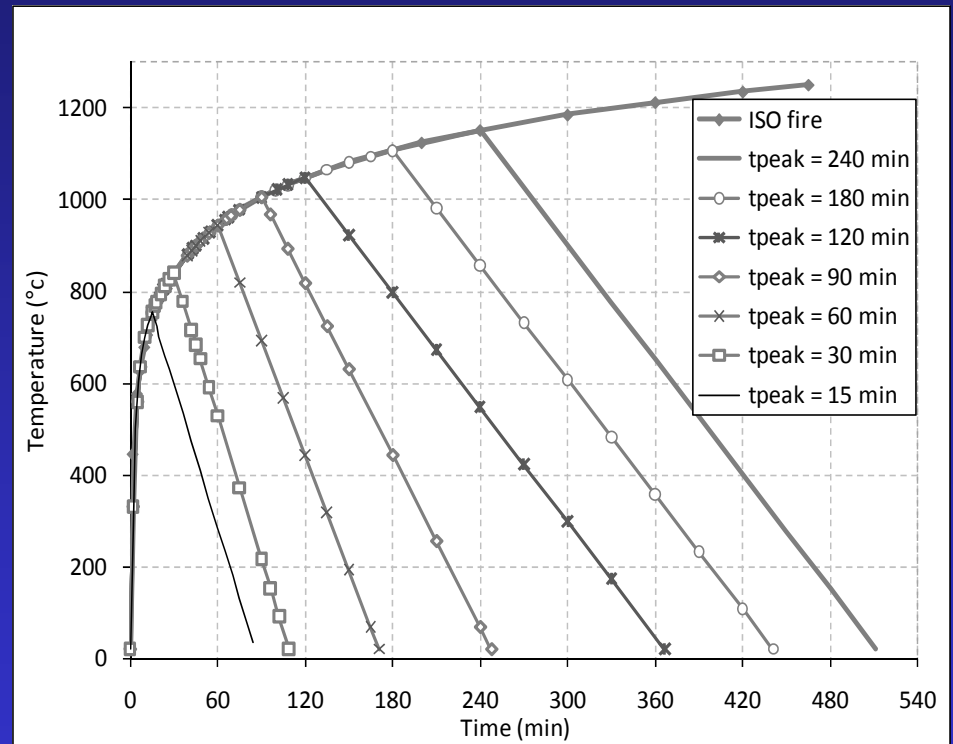
Non uniform temperature



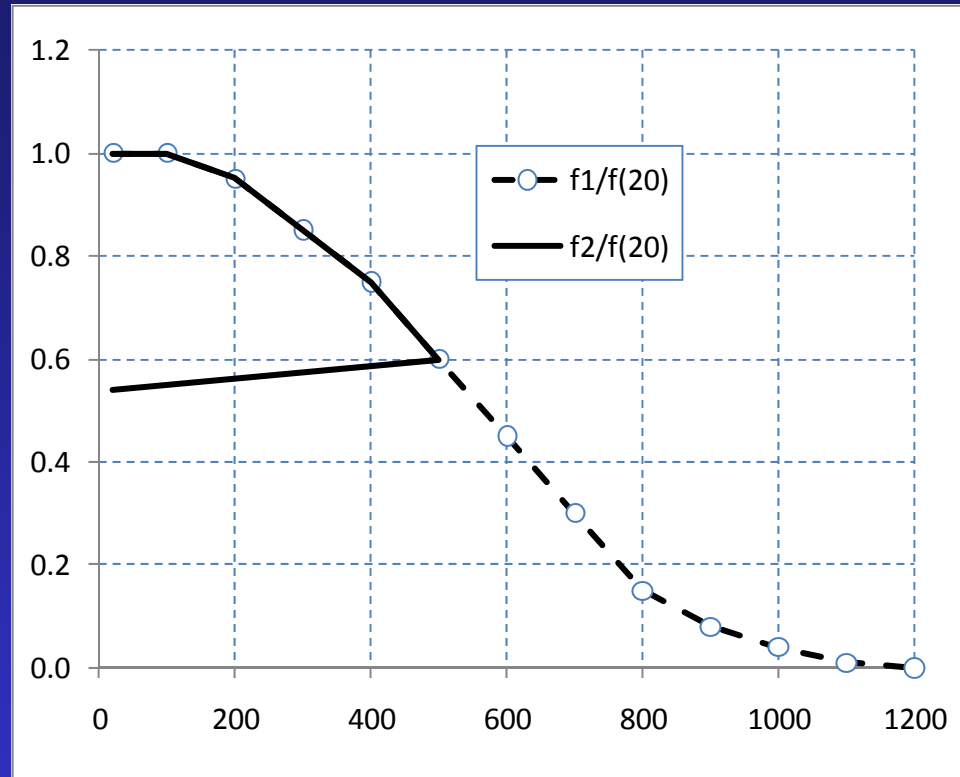
Yesterday
ISO fire

Method 4 : Advanced calculation models

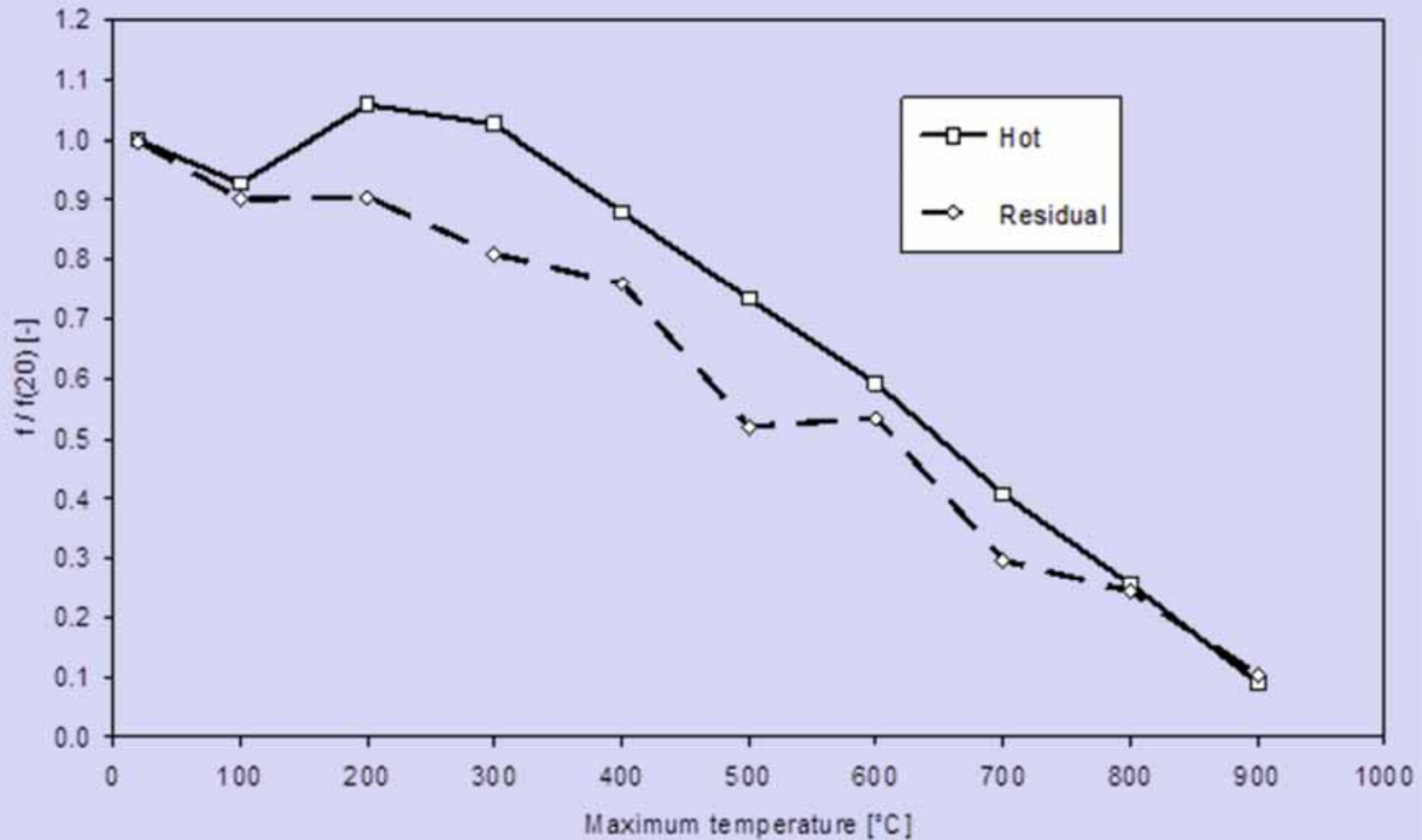
Today
Natural fires (with cooling phase)



Requires specific material models.



Compressive strength of concrete



Difference between hot and residual compressive strength
From Li & Franssen, Journal of Structural Fire Engineering, 2(1), 2011, 29-44.



Collapse of an underground car park after the fire has been put down³⁵

Yesterday

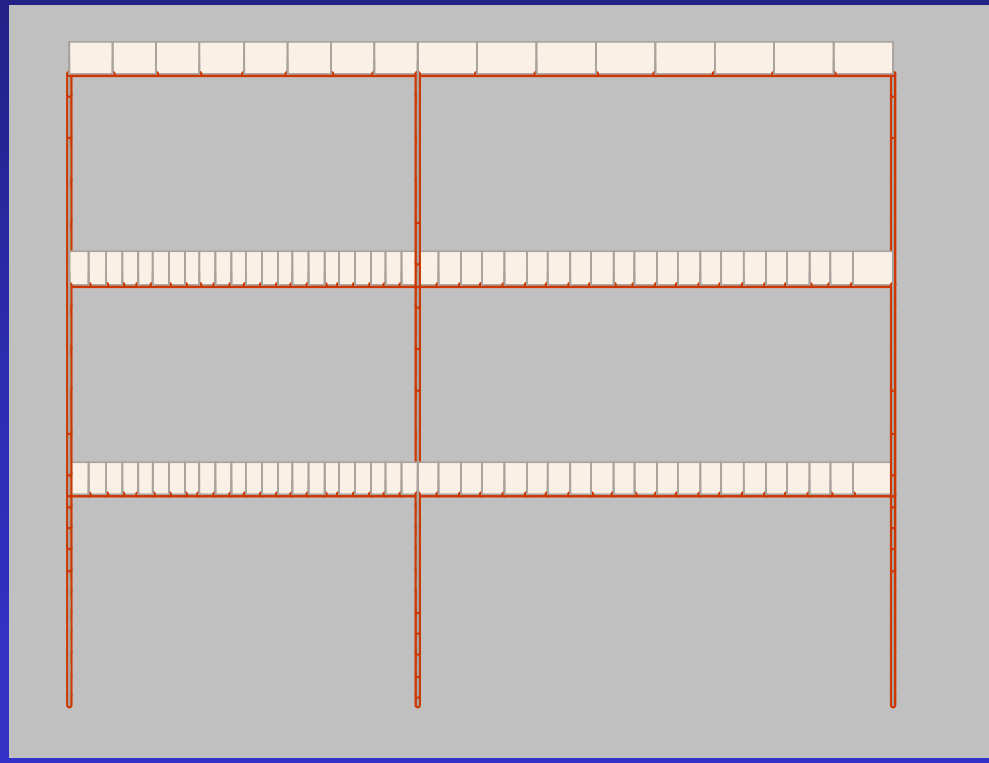
Implicit transient creep

Today

Explicit transient creep

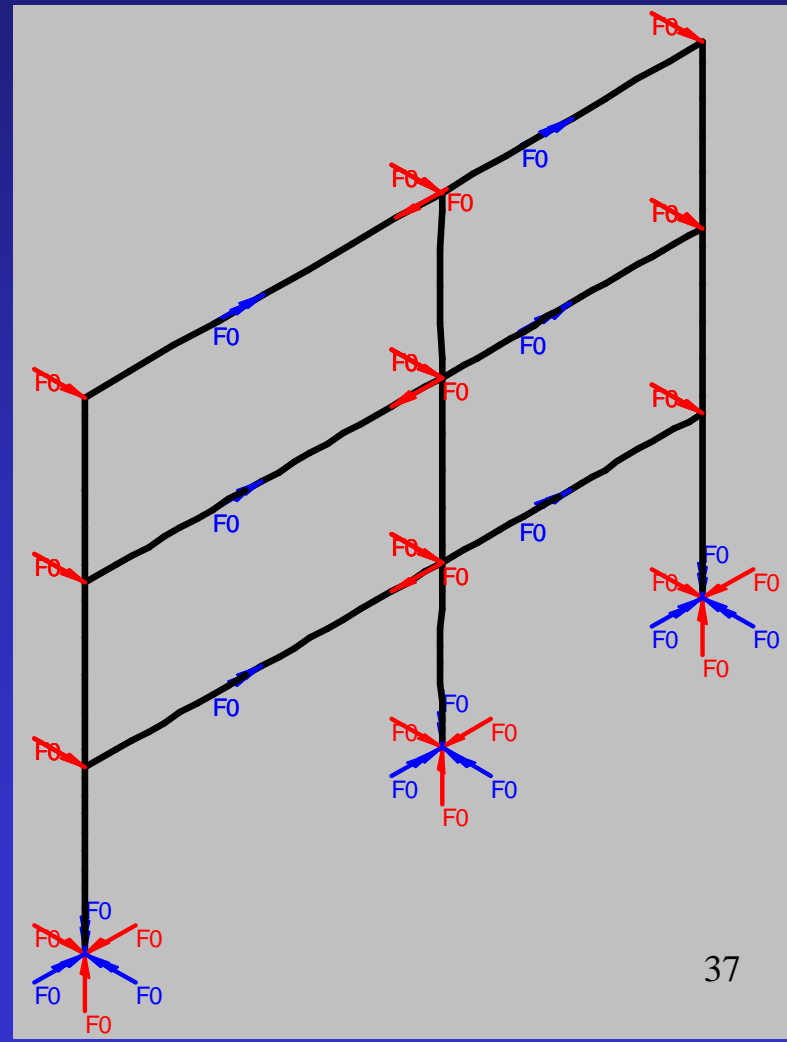
Yesterday

Single members or 2D frames

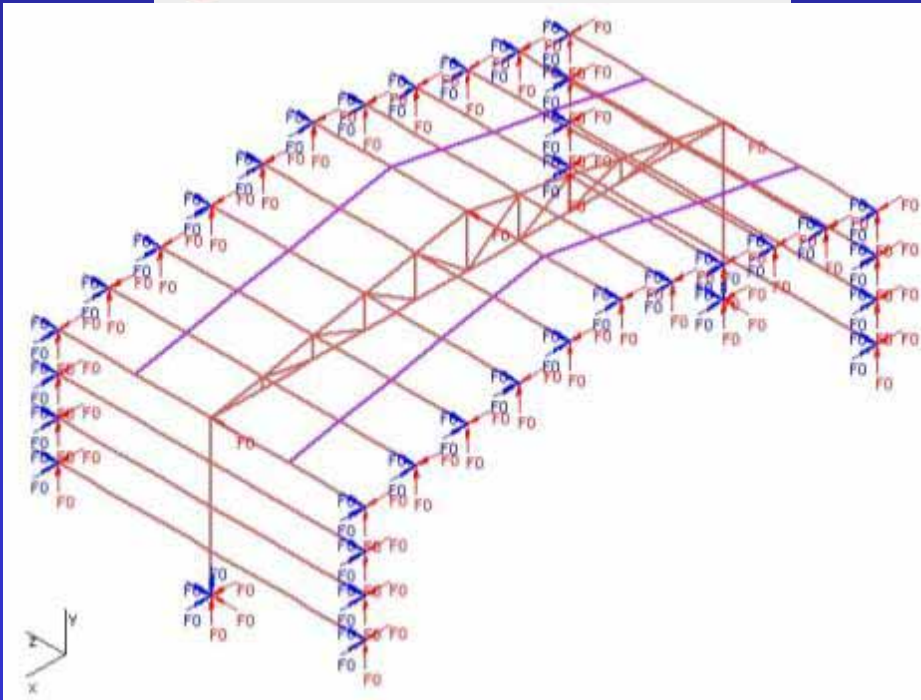
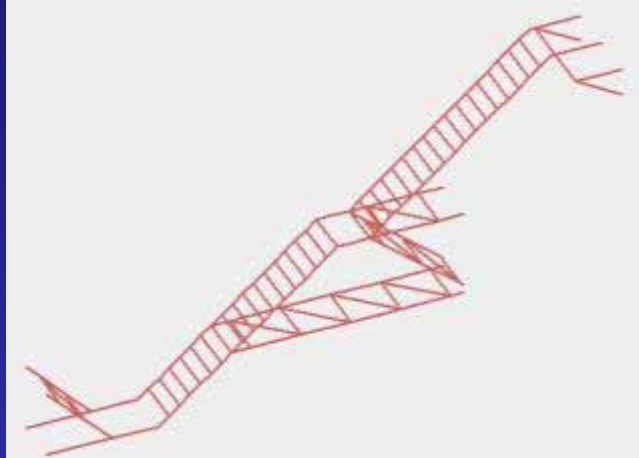


Today

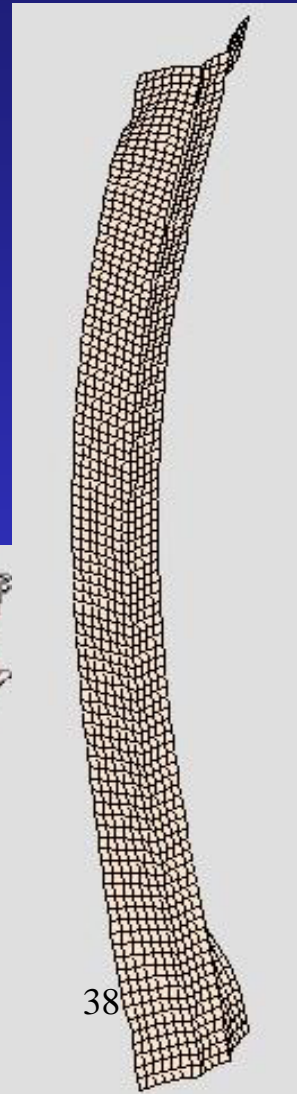
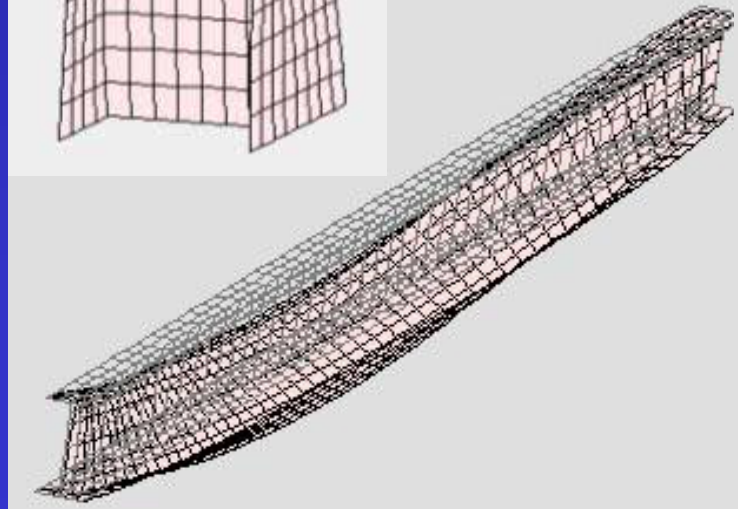
3D analyses



Yesterday Linear elements

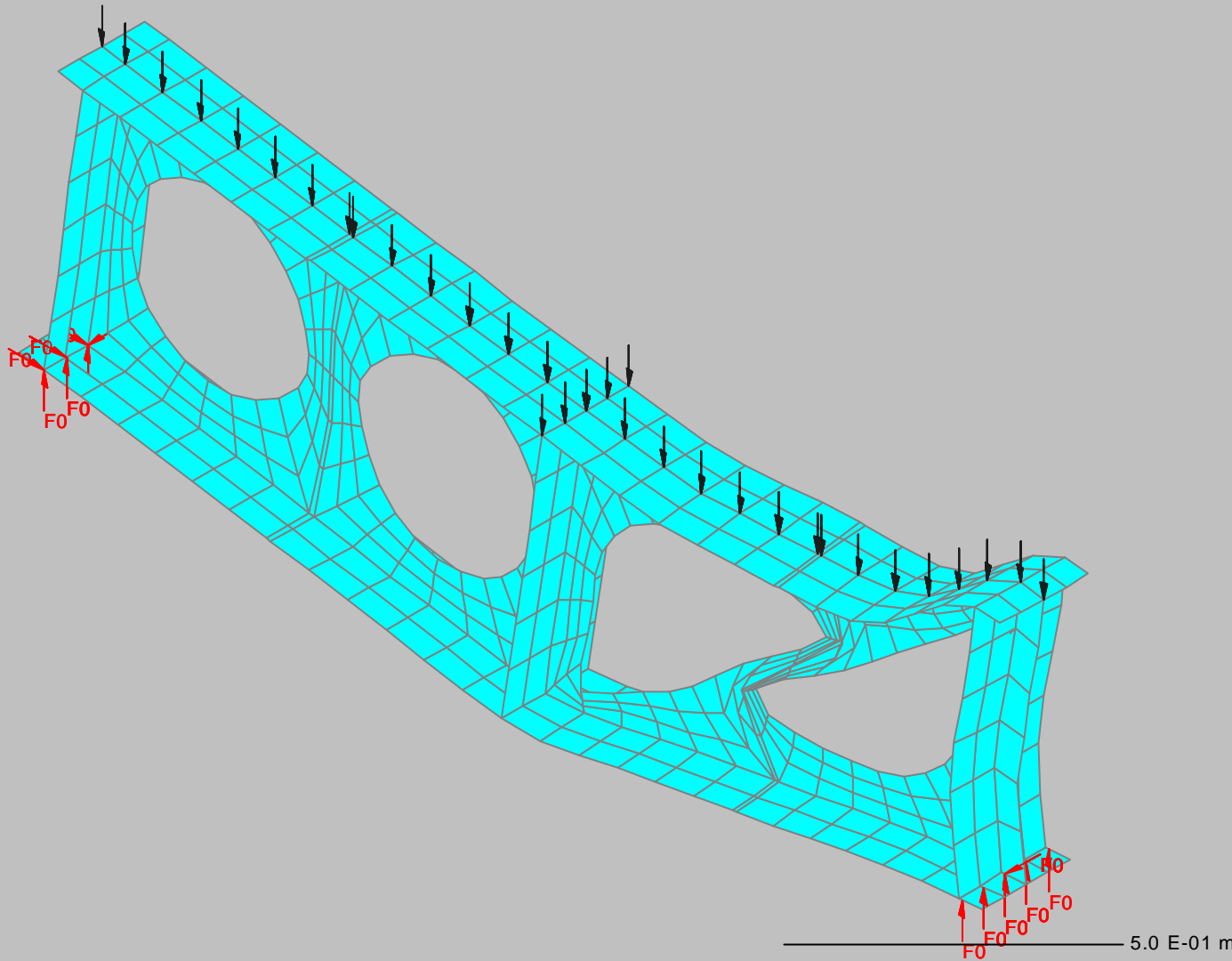


Today Shell elements



Short Cellular Steel beam

Symmetry not used



Diamond 2004 for SAFIR

FILE: acb_dyn_hot

NODES: 905

BEAMS: 0

TRUSSES: 0

SHELLS: 608

SOILS: 0

IMPOSED DOF PLOT

POINT LOADS PLOT

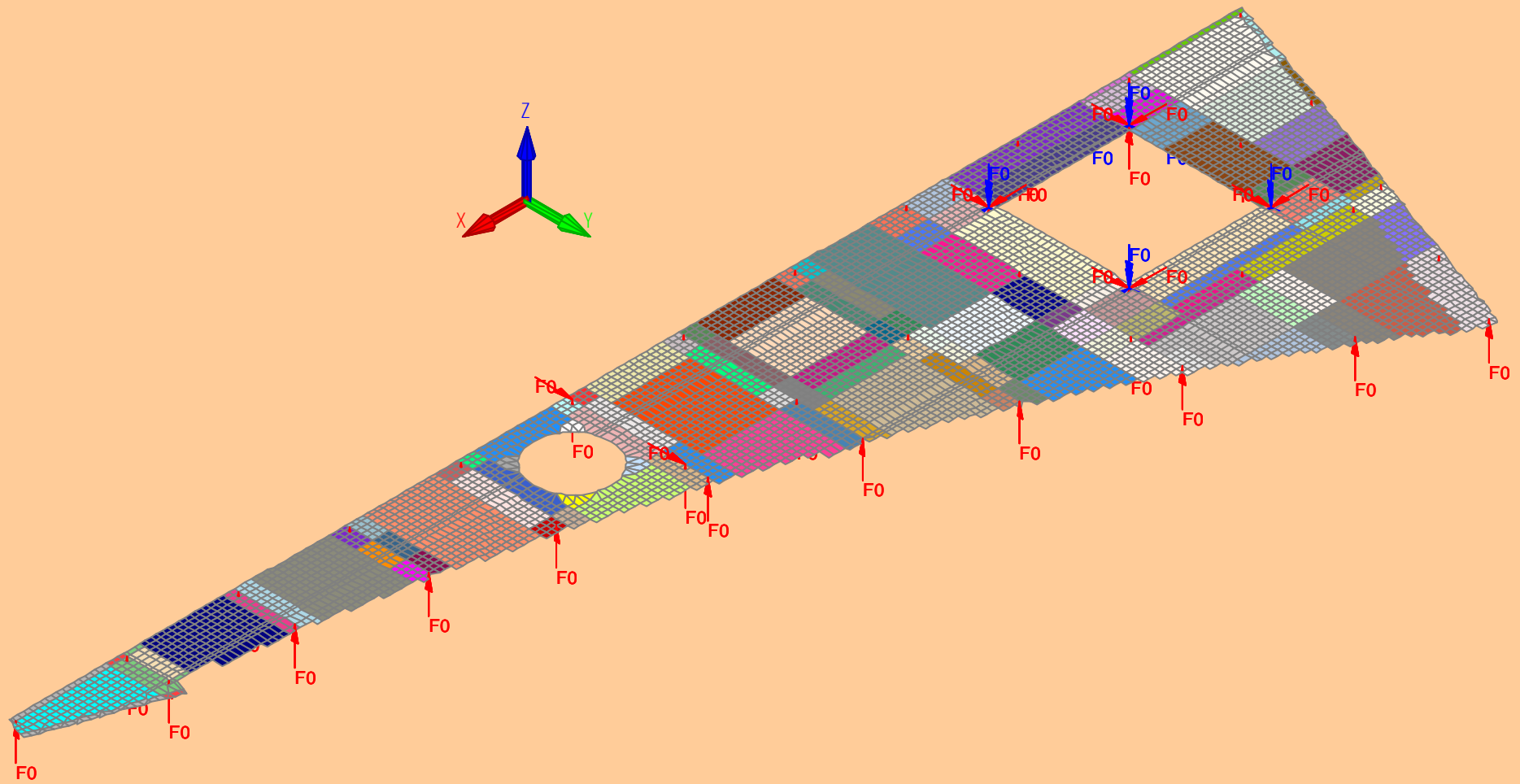
DISPLACEMENT PLOT (x 1)

TIME: 651.1728 sec

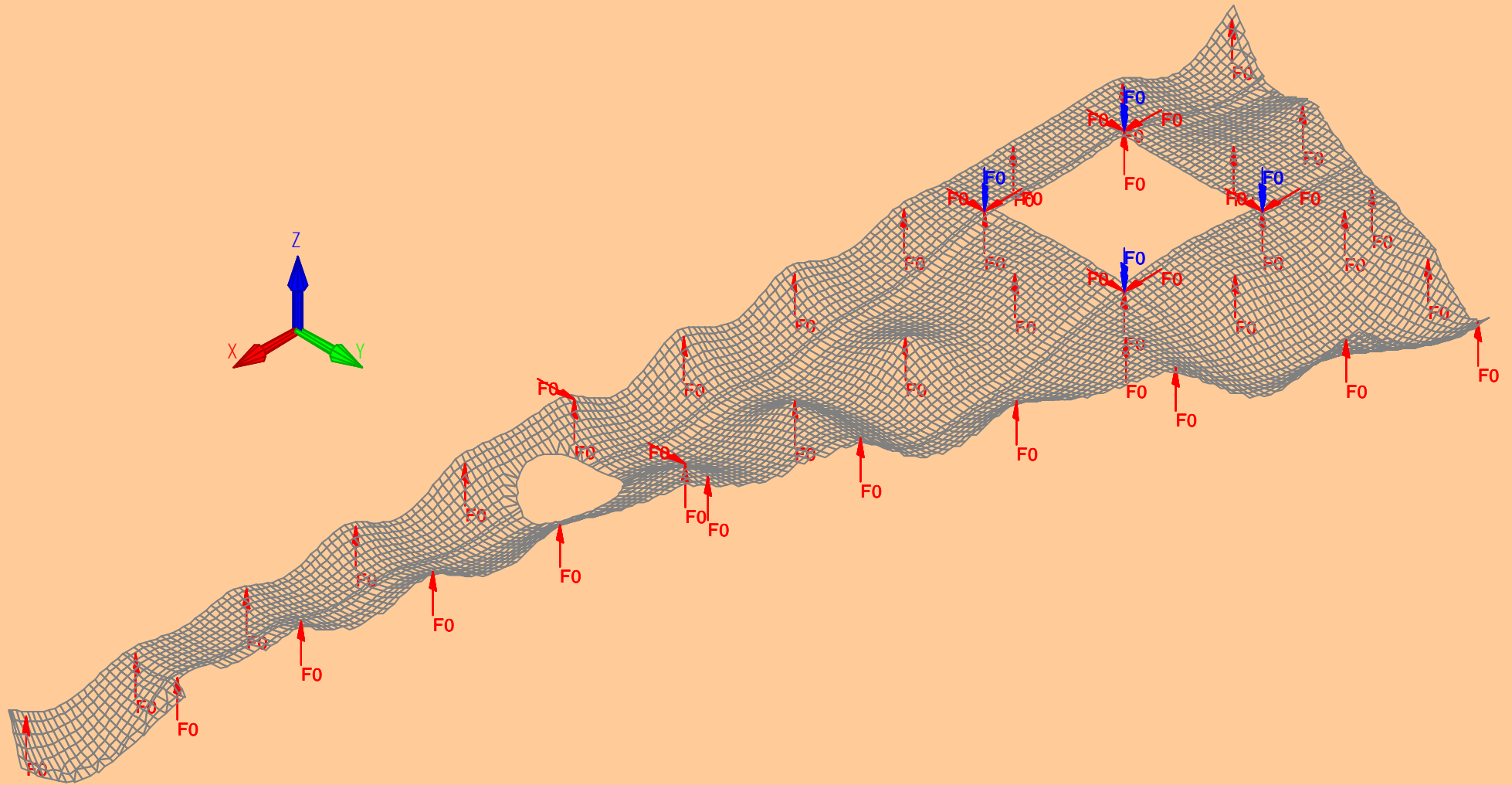
Reinforced concrete flat slab (20°C)



Courtesy "Batiserf, Grenoble"

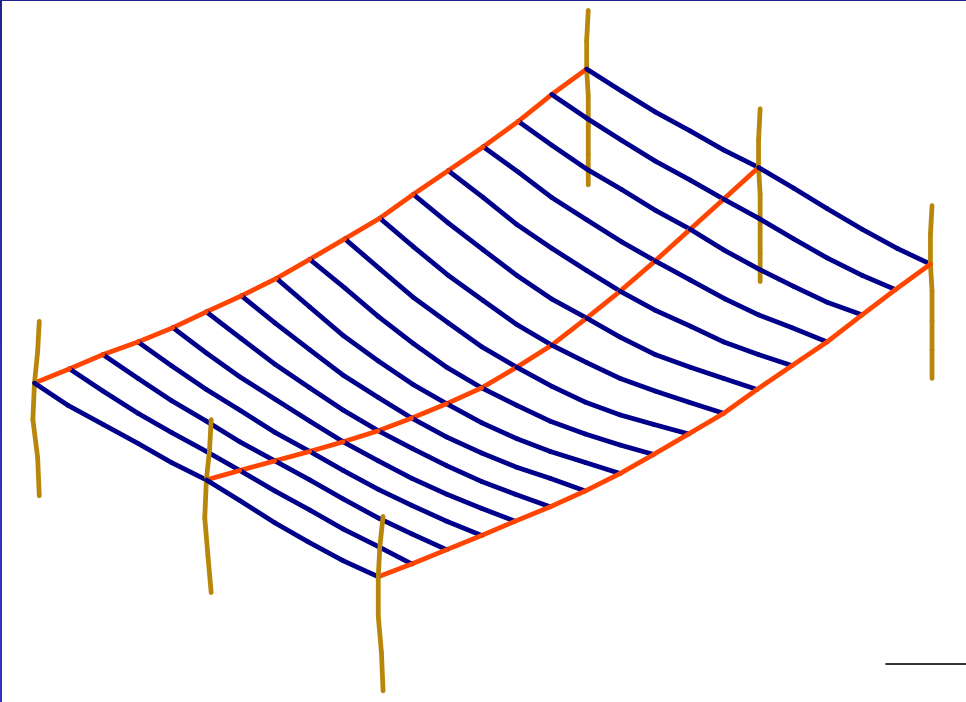


Displacement in the ultimate limit state



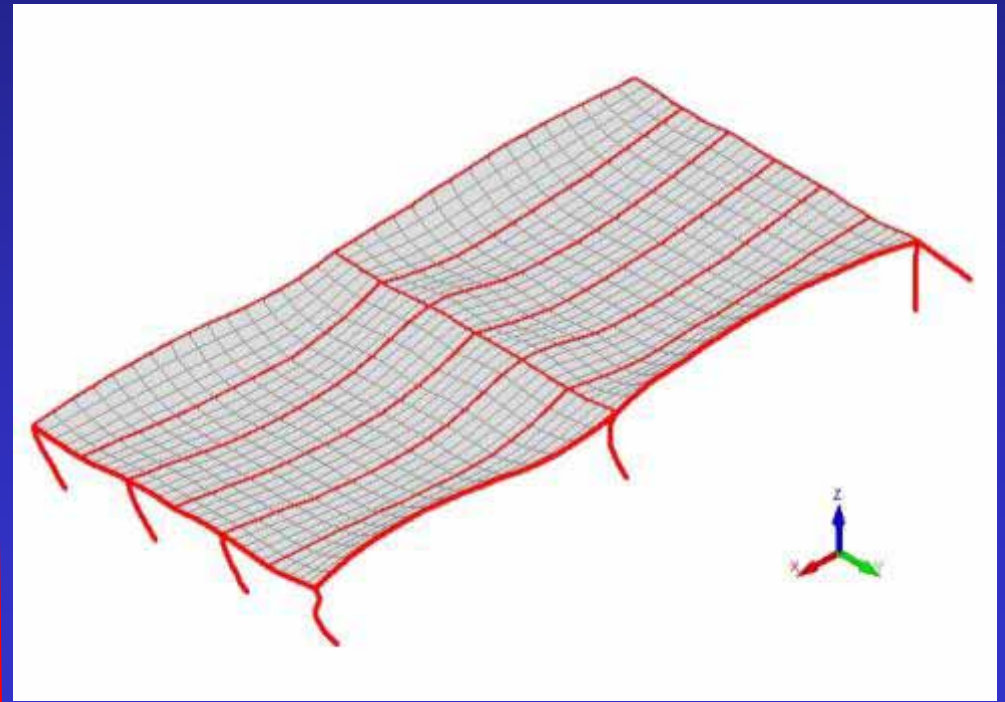
Yesterday

One type of F.E.



Today

Several types of F.E.

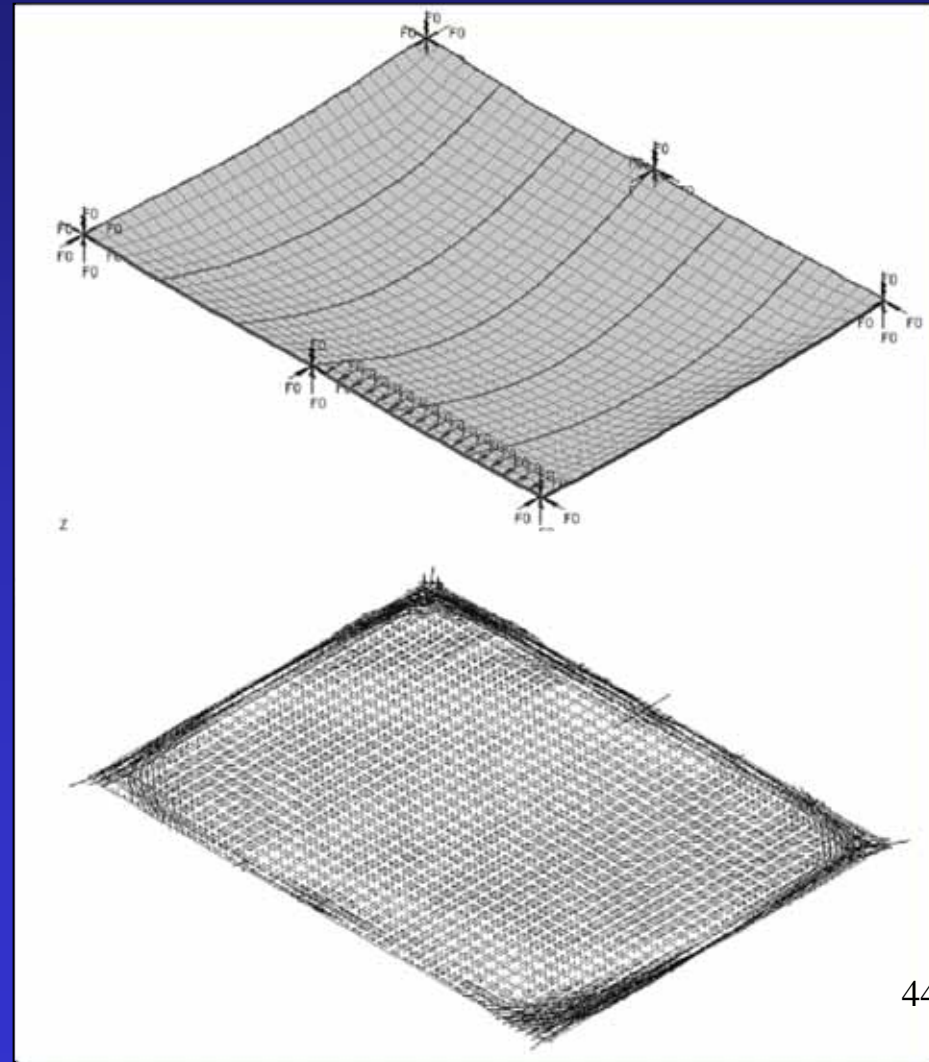


Yesterday

One way bending in floors

Today

Tensile membrane action



Yesterday

Static analyses

$$\{\Delta F\} = [K] \{\Delta u\}$$

Today

Dynamic analyses

$$\{F\} = [K] \{u\} + [C] \{\dot{u}\} + [M] \{\ddot{u}\}$$

Lee's Frame Analysed with Shell F.E. in bending

$dT/dt = 1^\circ\text{C/s}$



Diamond 2004 for SAFIR

FILE: Lee_flex_dyn_hot
NODES: 162
BEAMS: 0
TRUSSES: 0
SHELLS: 80
SOILS: 0

SHELLS PLOT
POINT LOADS PLOT
DISPLACEMENT PLOT (x 1)

TIME: 20.97152 sec

Lee_shell_hot.tsh

Other considerations

Failure mode may be more critical than time of collapse

Diamond 2004 for SAFIR

FILE: Frame stat 2D

NODES: 123

BEAMS: 61





TRUSSES: 0

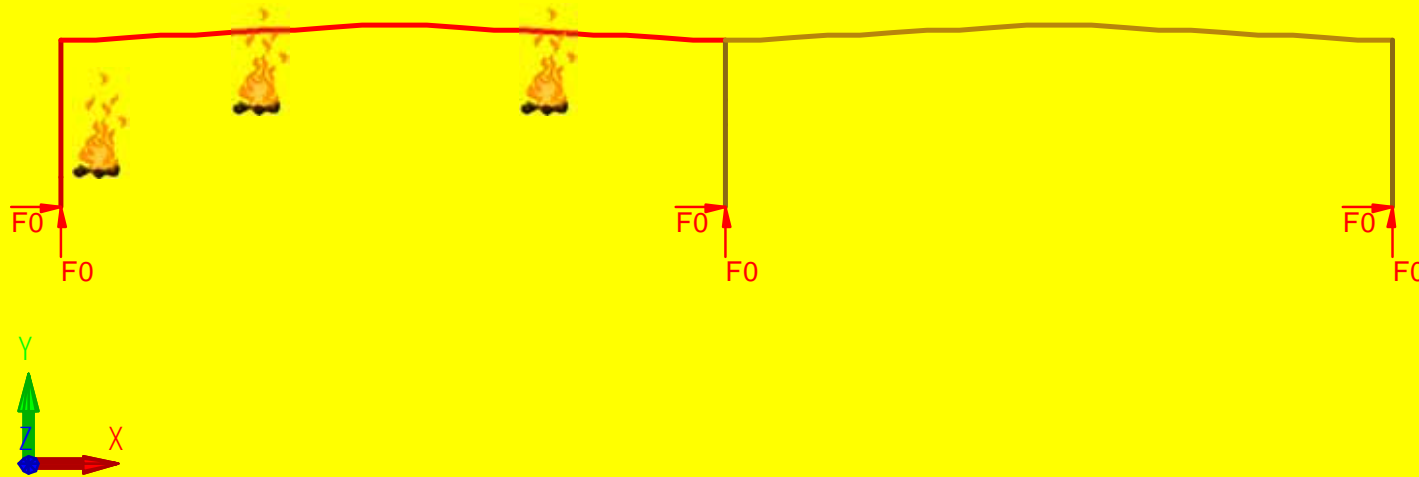
SHELLS: 0

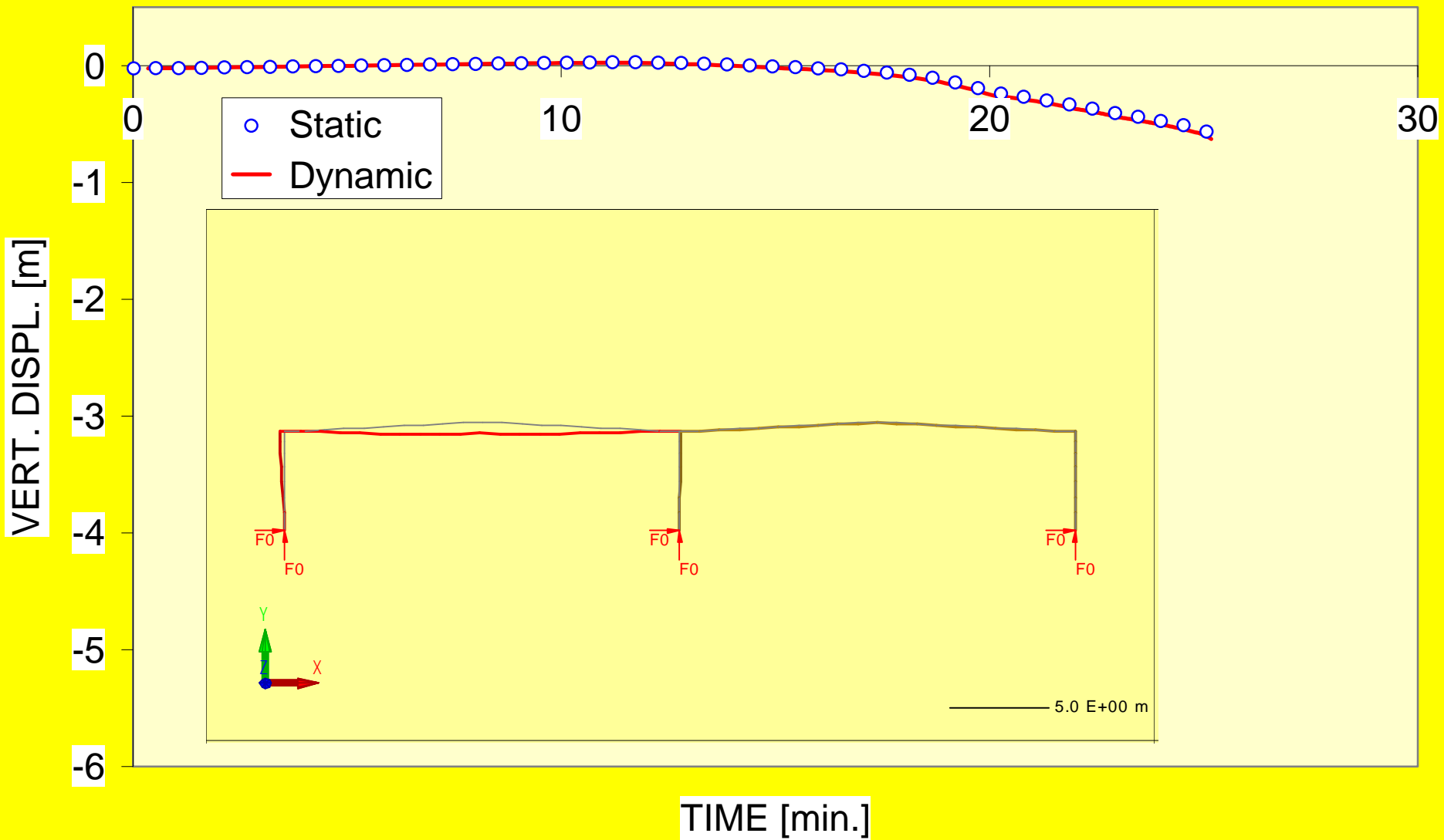
SOILS: 0

BEAMS PLOT

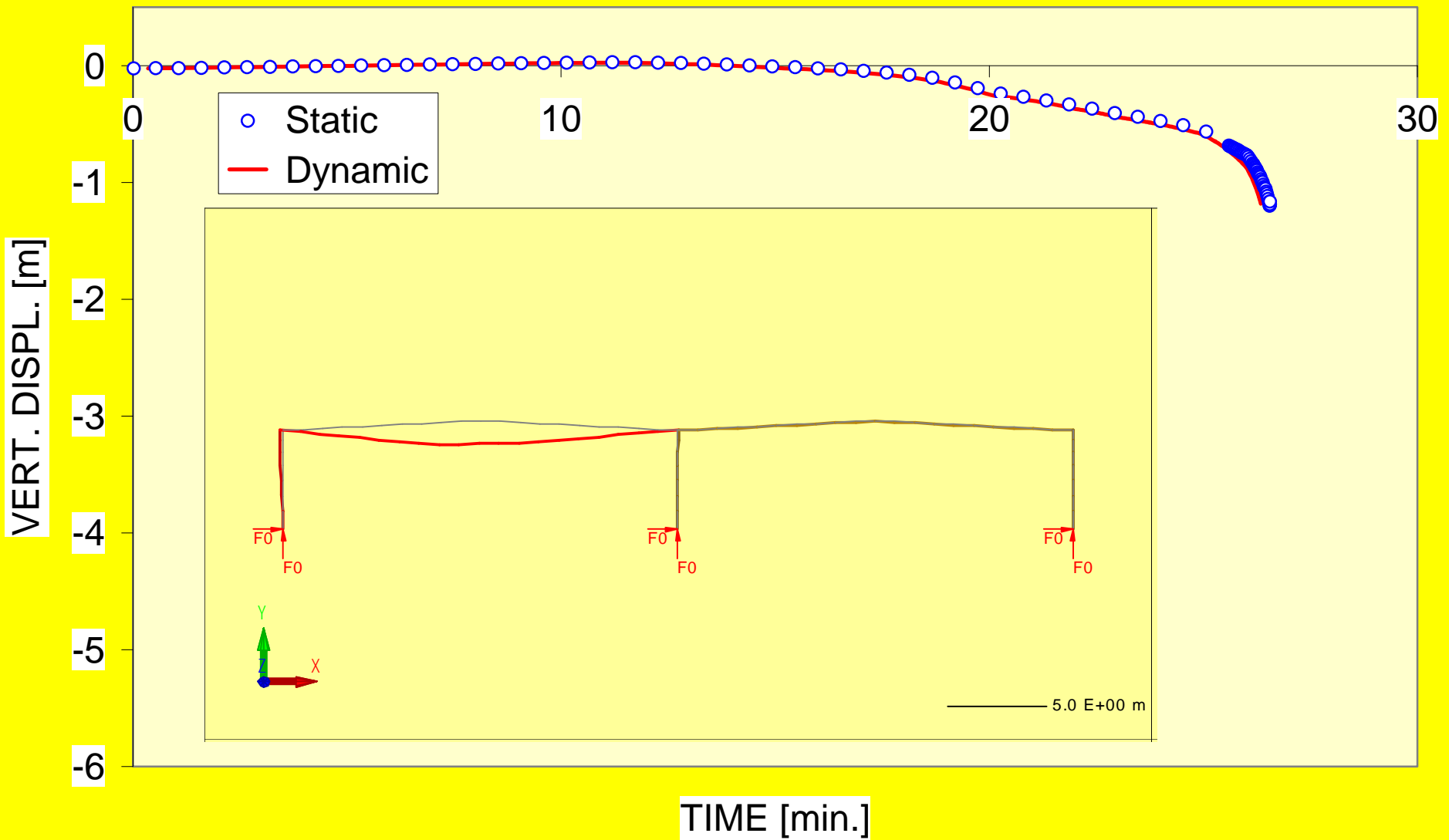
IMPOSED DOF PLOT

	IPE500.tem
	IPE450.tem
	IPE500c.tem
	IPE450c.tem

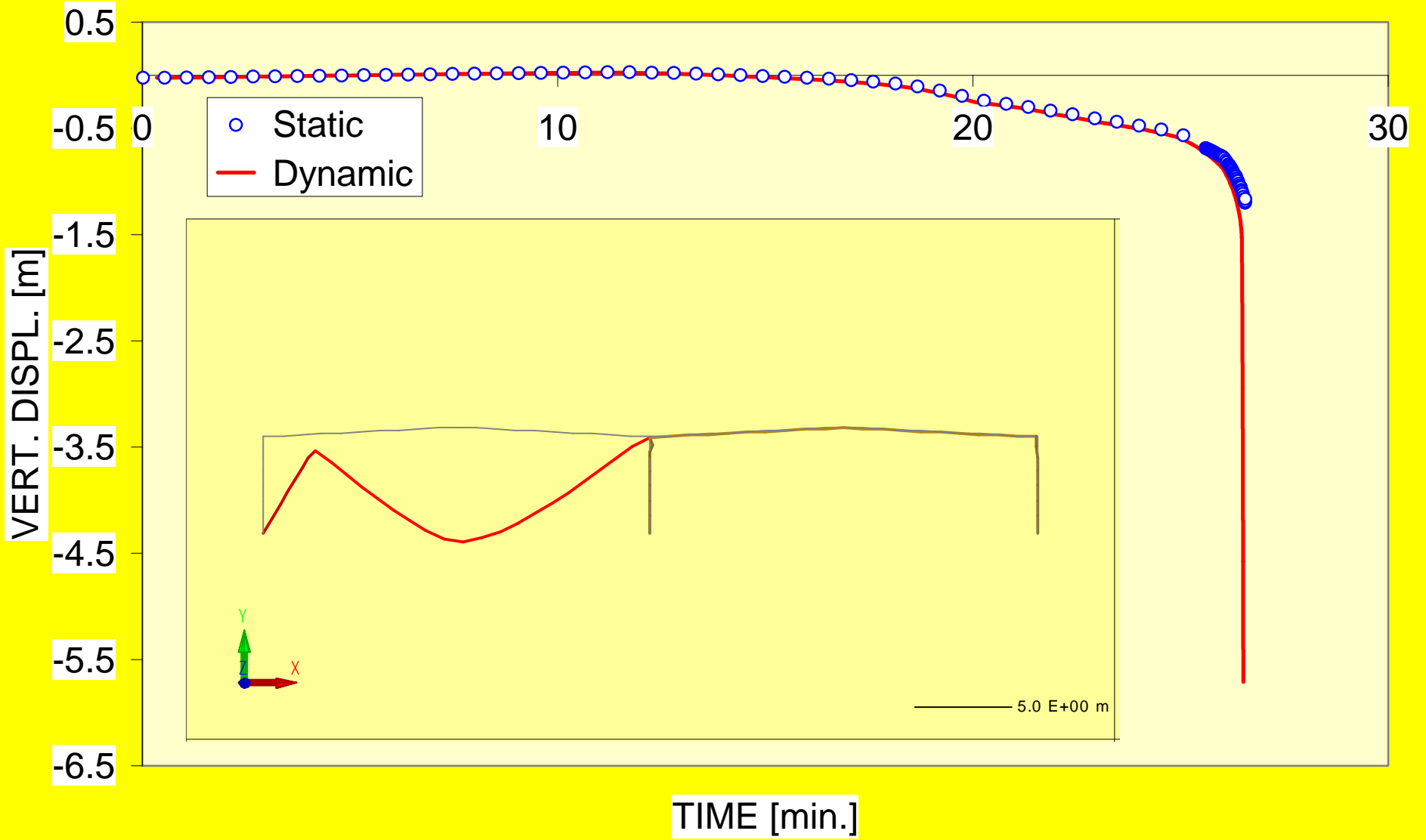




$t = 25'$



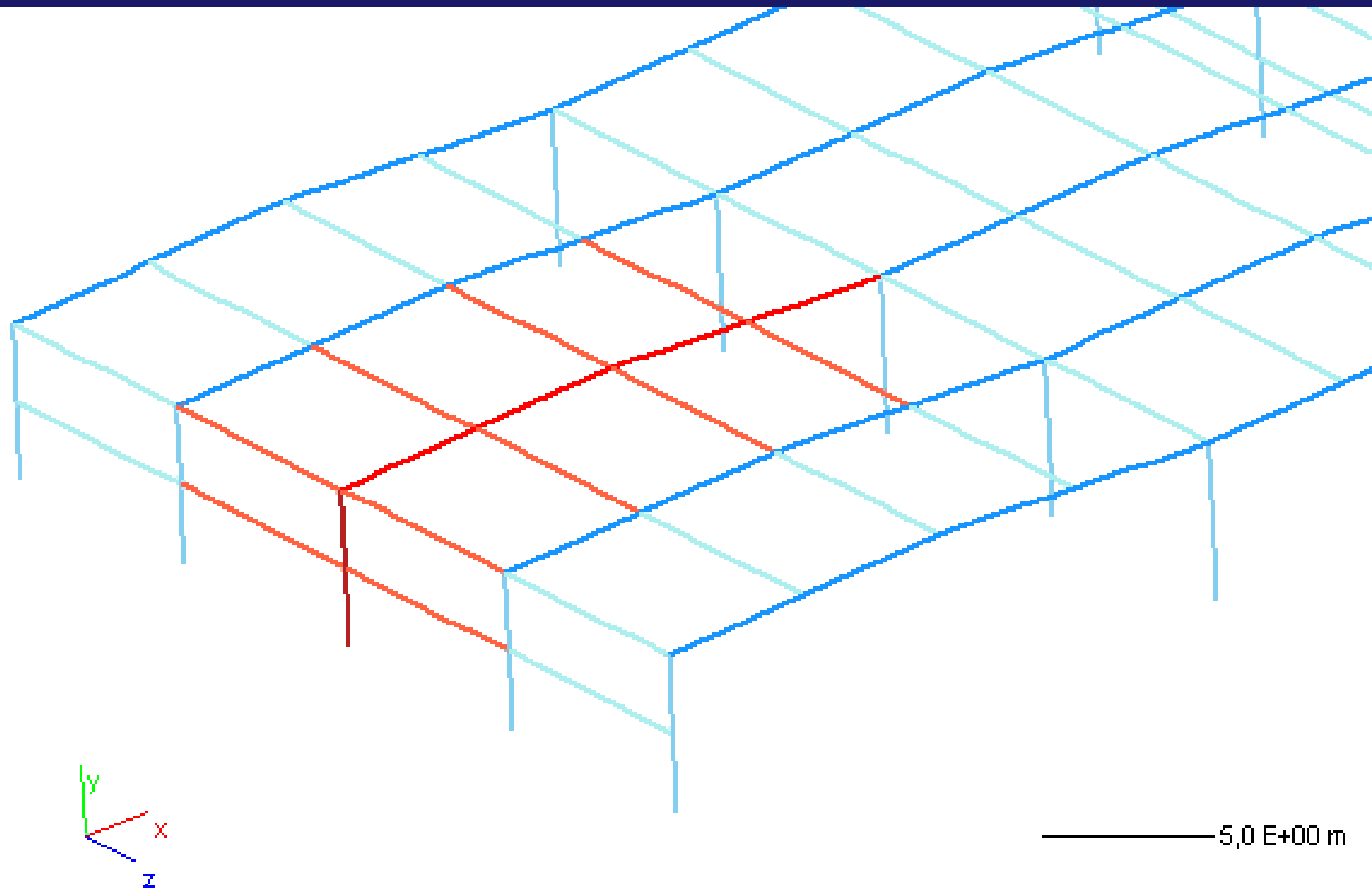
$t = 26'30''$



t = 26'34''



The same, now in 3D, with heated purlins



Diamond XL for S

LOCAL FIRE ON 3D

FILE: Animation

NODES: 1223

BEAMS: 585

TRUSSES: 0

SHELLS: 0

DISPLACEMENT PL

TIME: 20,97152 sec

3D frame (no amplification in the deformation)

When performing a S.i.F. analysis:

✓ make it simple,

✓ or not,

but not both.

Natural fire with cooling phase.

Criteria?

- ❖ Time of collapse (natural fire) $>$ required time for evacuation
- ❖ Infinite resistance (until complete burn out) ?
- ❖ Time of collapse (natural fire) = R(ISO) ?

Stupidity?

Or maybe not!

Representation of the fire ?

Nominal fire curve?

OK for structural research in the heating phase

Post-flashover parametric fire curve?

OK for structural research with a cooling phase

Zone models?

Ok if the geometry is appropriate

Difficulty for the columns in multi zone models

Representation of the fire ?

Local models (Hasemi)?

OK if the geometry is appropriate

Hasemi not applicable for columns

CFD

Not for post-flashover fires

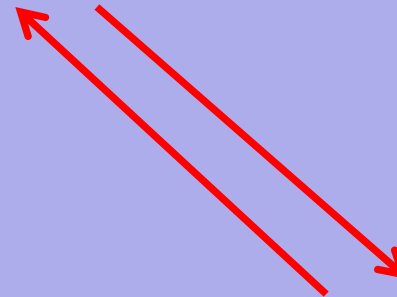
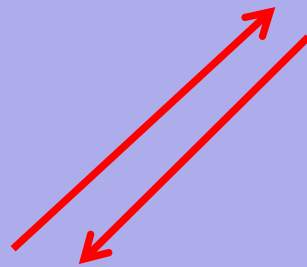
OK when local fire, large compartment with complex geometry, big budget.

Which interactions to consider?

IN REALITY EVERYTHING IS COUPLED



Temperatures in the structure



**Conditions
in the compartment**



Structural behaviour

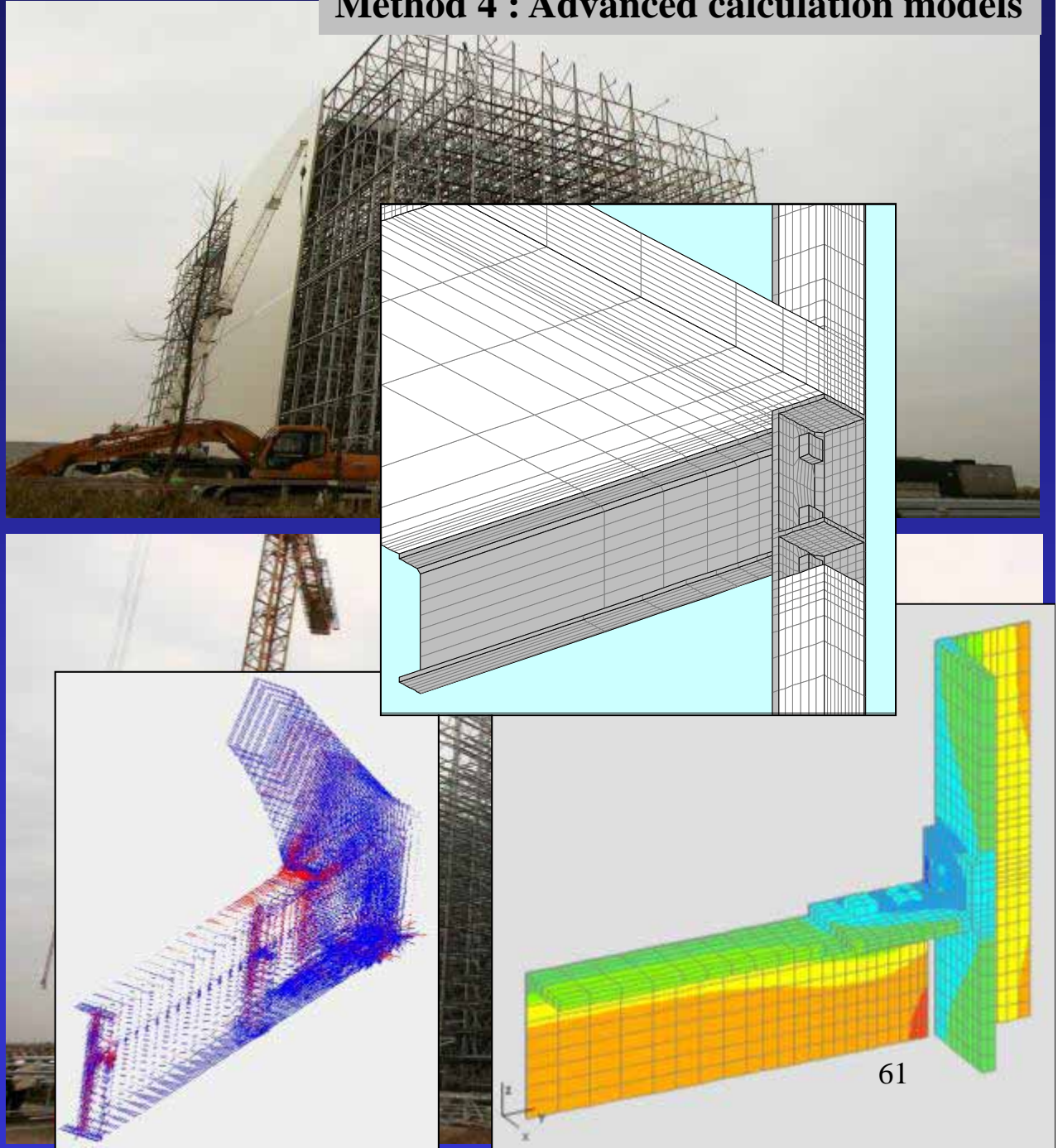
Structural fire engineering used in practice to:

- 1) Prove stability without any protection on steel
- 2) Reduce fire protection on steel
- 3) Prove fire resistance of existing concrete structure
- 4) Prove failure mode

Tomorrow?

- Very large models
- Connections
- Spalling of concrete

Method 4 : Advanced calculation models



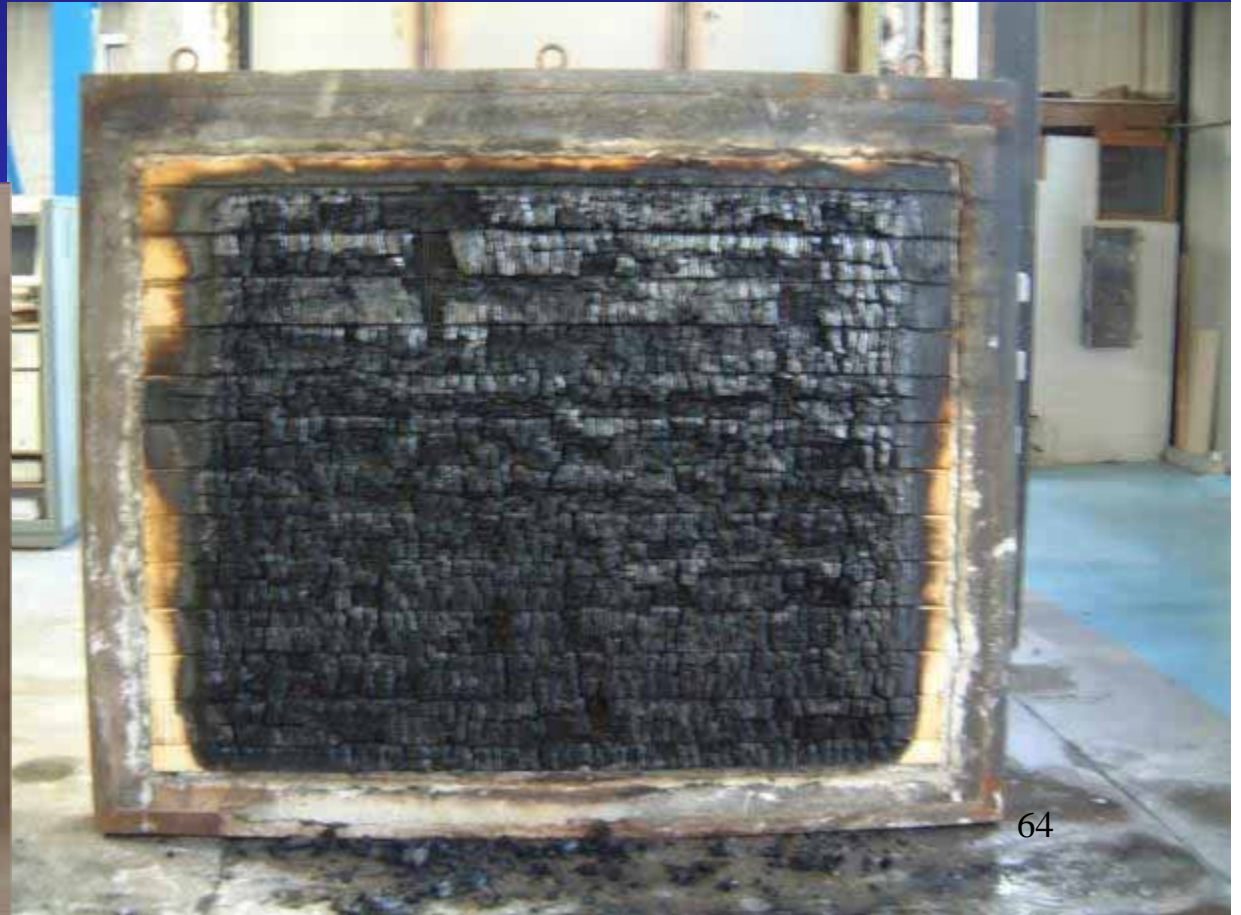


Tomorrow?

- **C.F.D. - F.E. interconnection**

Tomorrow

- **Moisture movements (e.g. in wood)**
- **Mechanical properties of gypsum**
- **Shear strength of concrete**
- ...





© the white planes picture co.

Thank you and Fly high!