

# WP4 –BENCHMARK STUDY : NUMERICAL BEHAVIOUR OF STEEL COLUMNS UNDER LOCALIZED FIRE LOADING

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## INTRODUCTION

**BENCHMARK STUDY:** Numerical model of a steel column under a natural fire

### **OBJECTIVE:**

Validation of the utilization of  
ABAQUS and SAFIR for steel  
structures under fire: thermal  
and mechanical analyses

against

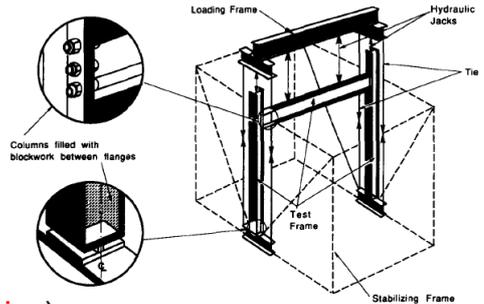
Experimental results and the  
numerical results obtained by  
the software CEFICOSS

**FIVE STUDY CASES:** the reference case  
the model definition  
axial restraint to beam  
frame continuity  
non-uniform temperature



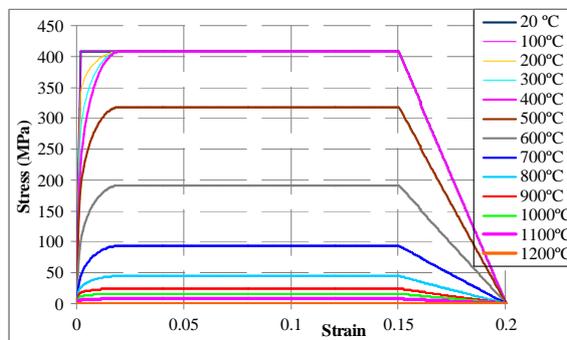
**BENCHMARK STUDY**

- ❑ PAPER PUBLISHED BY Franssen, Cooke and Latham (1995): Fully loaded, **2D** unprotected steel frame under a natural fire loading
- ❑ COLUMN: 3530mm long, Grade 43A ( $\approx$ S355), pin jointed at the base
- ❑ BEAM: 4550mm long, Grade 43A
- ❑ BOLTED BEAM-TO-COLUMN JOINTS
- ❑ LATERAL AND SWAY INSTABILITIES PREVENTED
- ❑ CONCRETE BLOCKS between the column flanges (**non-composite behaviour**)
- ❑ CONCRETE SLAB (**non-composite behaviour**)



**MATERIAL PROPERTIES**

- ❑ **STEEL PROPERTIES**  
Eurocode 3, part 1.2 (EN 1993-1-2:2005)  
At ambient temperature:  $f_y = 408 \text{ MPa}$ ;  $E = 210 \text{ GPa}$
- ❑ **CONCRETE PROPERTIES** (Slab + Blocks between column flanges):  
**Only plays an insulating role**

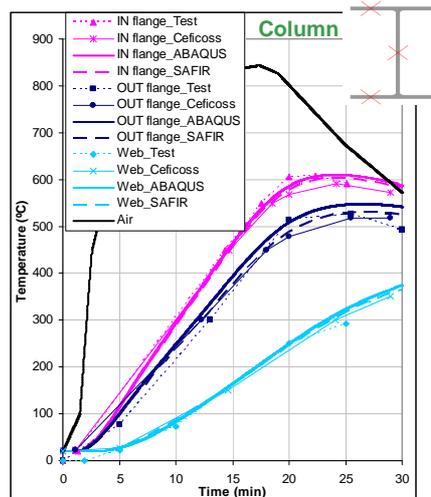
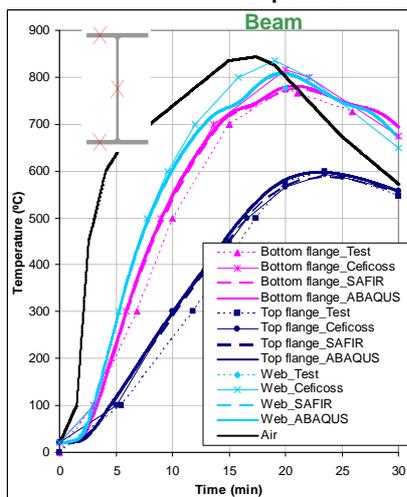


### THERMAL ANALYSIS

- **ANALYSIS** to determine the **distribution of temperatures** in each element
  - Involves **conduction and boundary radiation**
  - The **concrete slab** and the concrete blocks are modelled (thermal boundary conditions)
  - **2-D SOLID FE elements**

Mesh:	ABAQUS	SAFIR
Beam		
Column		

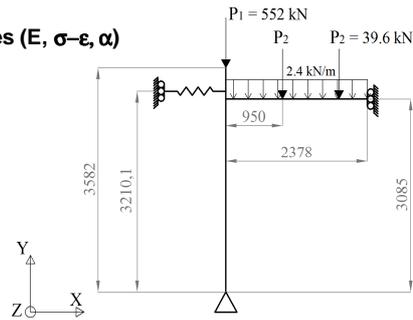
### THERMAL RESULTS: Good correlation between CEFICOSS, SAFIR, ABAQUS and experimental results



### MECHANICAL ANALYSIS

**GENERAL MODELLING ASSUMPTIONS**

- Static analysis
- 2D Beam elements
- Large-displacement effects and material nonlinearities
- Temperature dependent material properties ( $E, \sigma-\epsilon, \alpha$ )
- Nonlinear temperature gradient
- Concrete (slab + blocks) not modelled
- Rigid beam-to-column connection
- Bi-linear spring (restraint – 2<sup>nd</sup> steelwork)

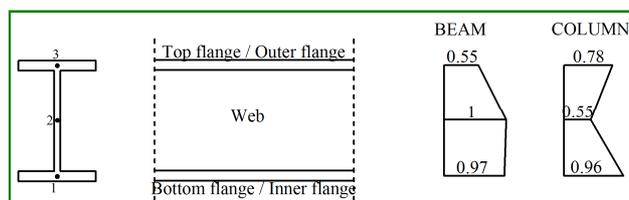


### MECHANICAL ANALYSIS

**MECHANICAL LOADING:** (i) applied at room temp, (ii) maintained constant (fire)

**THERMAL GRADIENT THROUGH THE SECTION**

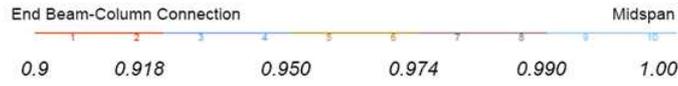
- **CEFICOSS, SAFIR:** calculated temp. (thermal analysis) used by the structural part
- **ABAQUS:** 3 Dependent temperatures at three points



### MECHANICAL ANALYSIS

☐ **THERMAL GRADIENT ALONG THE BEAM SPAN** (lower temp. at joint)

- **CEFICOSS, SAFIR**: sinusoidal function



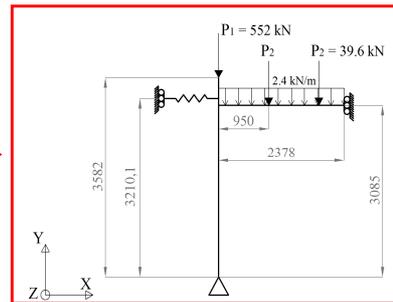
- **ABAQUS**: temperature along each length part is constant

Beam-to-column connection				Midspan
0.918 $\theta_a$	0.95 $\theta_a$	0.974 $\theta_a$	0.99 $\theta_a$	1.00 $\theta_a$

### MECHANICAL RESULTS

☐ **STUDY CASES**

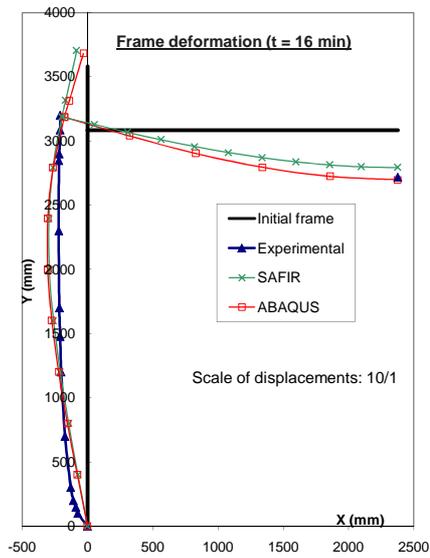
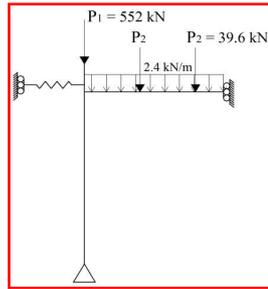
1. Reference case
2. Model definition
3. Axial restraint to beam
4. Frame continuity
5. Non-uniform distribution of temperature



**MECHANICAL RESULTS**

□ **1. REFERENCE CASE**

Lateral deformation influenced by the **concrete blocks** inserted between column flanges

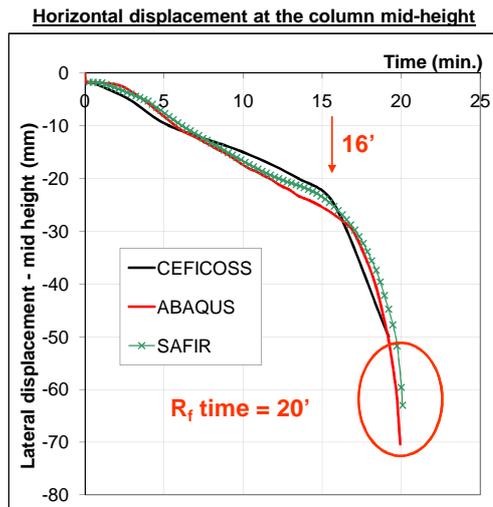


**MECHANICAL RESULTS**

□ **1. REFERENCE CASE**

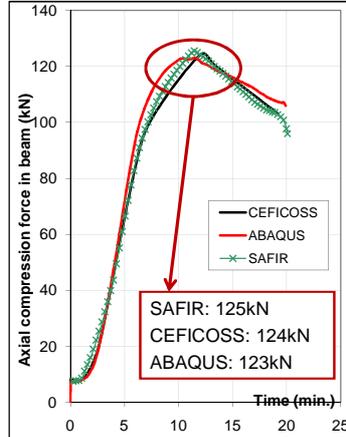
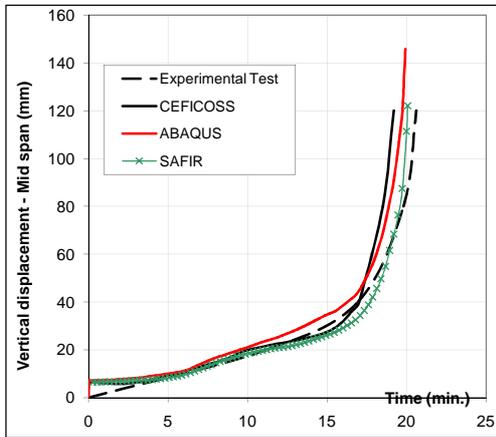
Due to the elongation of the beam → the **column bows laterally** up to the buckling around 19-20 min

Differences between programs: **Slight ≠ temperature gradients** in cross-sections



**MECHANICAL RESULTS**

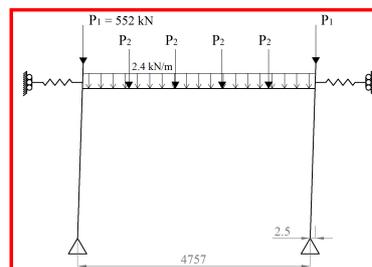
**1. REFERENCE CASE - Beam mid-span vertical displacement**



**MECHANICAL RESULTS**

**2. INFLUENCE OF THE MODEL DEFINITION**

- Initial lateral imperfection:  $0.8H_c/1000$
- Fire resistance time:
  - Good agreement between CEFICOSS and ABAQUS
  - ≠ SAFIR → difficulty in modelling the two springs working together

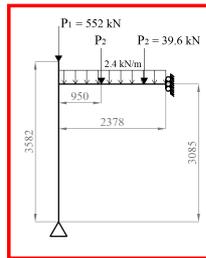


	R <sub>f</sub> CEFICOSS	R <sub>f</sub> SAFIR	R <sub>f</sub> ABAQUS
1 - half of the frame	19'12"	20'04"	19'55"
2 - complete frame	19'22"	17'55"	19'51"

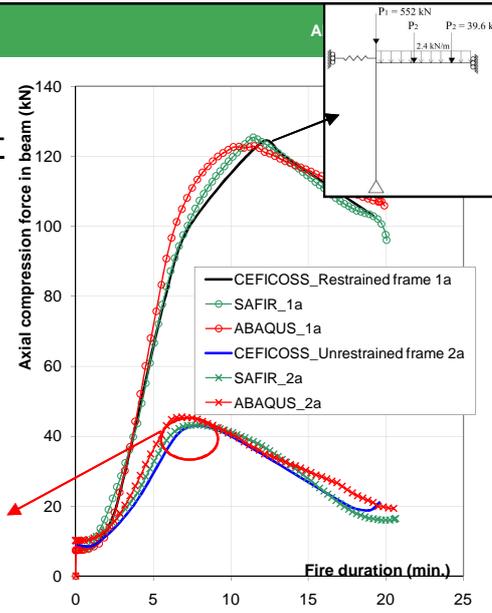
**MECHANICAL RESULTS**

**3. INFLUENCE OF THE AXIAL RESTRAINT**

- GOOD CORRELATION
- Axial compression force ↘
- Fire resistance and Stability few affected



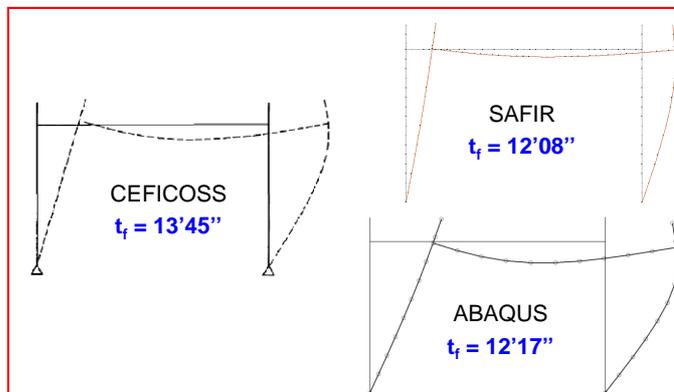
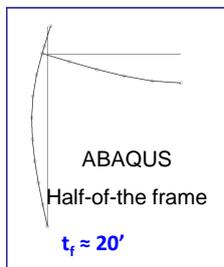
ABAQUS: 45kN  
SAFIR: 43kN  
CEFICOSS: 43kN



**MECHANICAL RESULTS**

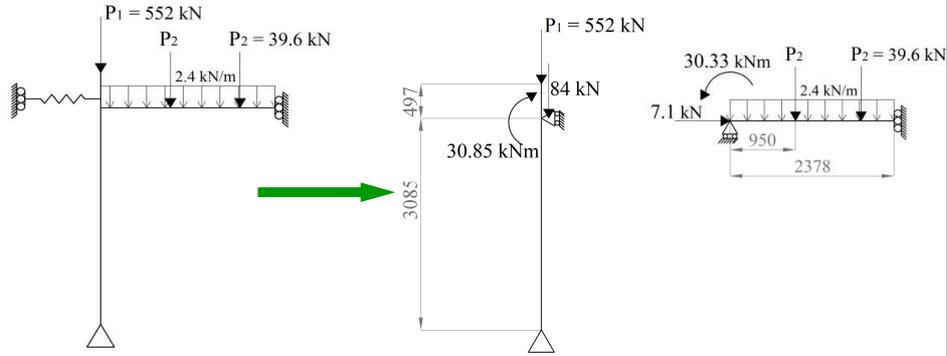
**3. INFLUENCE OF THE AXIAL RESTRAINT TO BEAM – Complete frame**

- ≠ failure mode (sway) → Fire resistance reduced from 20 min. to 12-13 min.
- Good correlations between numerical programs



**MECHANICAL RESULTS**

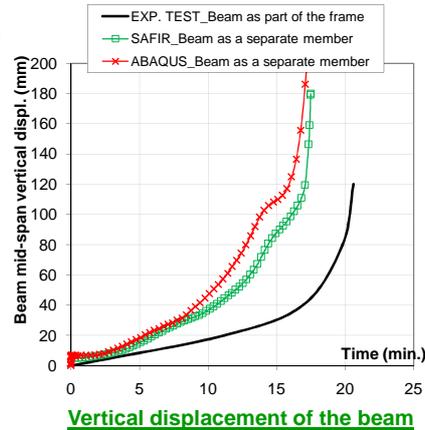
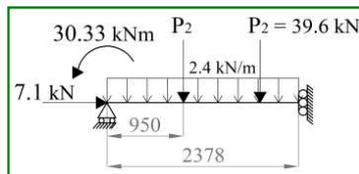
4. INFLUENCE OF THE FRAME CONTINUITY



**MECHANICAL RESULTS**

4. INFLUENCE OF THE FRAME CONTINUITY – Beam on its own

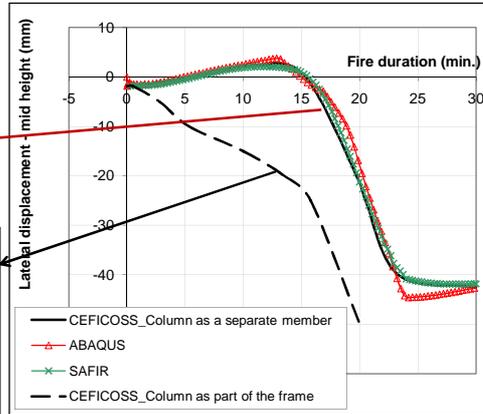
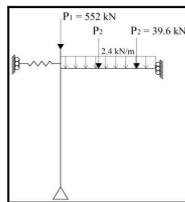
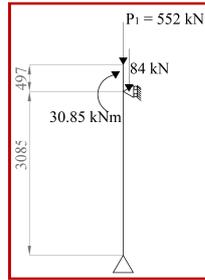
- Vertical displacement ↑ (no beneficial restraints from the column)
- Reduction of the  $R_f$  time (17'-18' for SAFIR and ABAQUS)
- Small Discrepancies between FE programs SAFIR and ABAQUS



### MECHANICAL RESULTS

4. INFLUENCE OF THE FRAME CONTINUITY – Column on its own

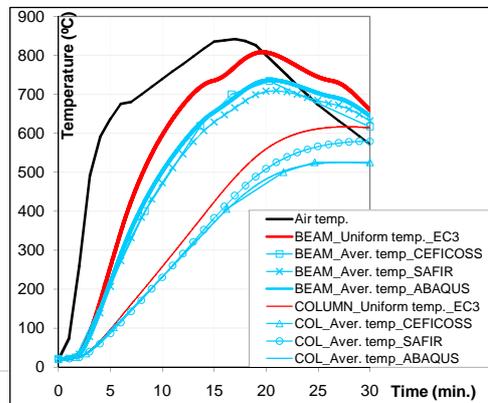
- All the programs:  $R_f$  time > 30' ( $\neq$  20min.) **Horizontal displacement of the column**
- Column on its own: behaviour  $\neq$
- Column remains stable - no collapse



### MECHANICAL RESULTS

5. INFLUENCE OF THE NON-UNIFORM TEMPERATURE

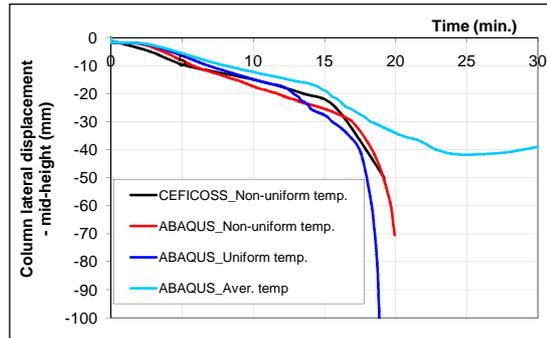
- Gradient temp. (reference case)
- Uniform Temp through the cross-section: EN 1993-1-2:2005 - simplified method
- Average temperature



## **MECHANICAL RESULTS**

### **5. INFLUENCE OF THE NON-UNIFORM TEMPERATURE - Column**

- **Uniform temperature** (EC3): leads to *conservative results* (premature failure)
- **Average temperature**: leads to *unsafe results* (no failure is observed until the end of the analysis)



## **CONCLUDING REMARKS**

- SAFIR and ABAQUS performed well the *heat transfer analysis***
- SAFIR and ABAQUS showed a good ability to simulate steel structural behaviour under fire conditions using beam elements**
- Some of the differences between the results of the 2 programs could be explained by:**
  - Temperature gradients in cross-sections approximated in ABAQUS
  - Difficulty to simulate the behaviour of springs with SAFIR (no dedicated finite element)



